

Memo

Bridge Office Mail Stop 610 3485 Hadley Avenue North Oakdale, MN 55128-3307 Phone No: 651/366-4500 Fax No: 651/366-4497

April 24, 2013

To:

Tony Schrempp

MnDOT Central Resident Construction Office

From:

Arielle Ehrlich HGE

State Bridge Design Engineer

Bridge Office

Subject:

S.P. 2774-27504A

Bridge No. 27504 Revised Plan Sheets

Enclosed is the revised plan sheet 9-R2 for Bridge 27504. The deflection diagram for Beam 7 was flipped between spans, and this revision was made to reflect the correct deflection diagram. This revision will not affect the cost for this project.

Copies of prints sent to:

- Tony Schrempp (1)
- Paul Pilarski (1)
- Ben Jilk (1)
- Bridge Office File (1)

Encl: Copies of prints as noted.

c.c.: N. Daubenberger / A. Ehrlich

J. Southward



Memo

Bridge Office Mail Stop 610 3485 Hadley Avenue North Oakdale, MN 55128-3307 Phone No: 651/366-4500 Fax No: 651/366-4497

September 13, 2013

To:

Steve Barrett

Resident Engineer Golden Valley

From:

Arielle Ehrlich

State Bridge Design Engineer

Bridge Office

Subject:

S.P. 2774-16

Bridge No. 27504 Revised Plan Sheets

Enclosed are the revised plan sheets 16-R2, 17R and 18-R2 for Bridge 27504. These revisions were made to clarify section E-E on sheet 16-R2, to change slope of Mock-Up panel on sheet 17R and to change Construction Sequence on sheet 18-R2. These revisions will not affect the cost for this project.

Copies of prints sent to:

- Steve Barrett (3)
- Ron Rauchle (1)
- Paul Pilarski (1)
- Ben Jilk (1)
- Bridge Office File (1)

Encl: Copies of prints as noted.

c.c.: N. Daubenberger / A. Ehrlich

J. Southward



Memo

Bridge Office Mail Stop 610 3485 Hadley Avenue North Oakdale, MN 55128-3307 Phone No: 651/366-4500 Fax No: 651/366-4497

July 30, 2013

To:

Steve Barrett

Resident Engineer Golden Valley

From:

Arielle Ehrlich ALGE

State Bridge Design Engineer

Bridge Office

Subject:

S.P. 2774-16

Bridge No. 27504 Revised Plan Sheets

Enclosed are the revised plan sheets 1-R2, 2-R2, 3-R2 and 23-R1 for Bridge 27504. These revisions were made to add conduit through the sidewalk on the bridge. Since this adds a pay item, this revision will affect the cost for this project.

Copies of prints sent to:

- Steve Barrett (3)
- Ron Rauchle (1)
- Paul Pilarski (1)
- Ben Jilk (1)
- Bridge Office File (1)

Encl: Copies of prints as noted.

c.c.:

N. Daubenberger / A. Ehrlich

J. Southward

Addendum 1 for changes to the Plans and Special Provisions for Bridge Nos. 7268 and 27504:

Plans for Bridge No. 7268:

- 1) On Sheet 1R of 22 for Bridge No. 7268 the following changes were made:
 - a. Notes and call-outs were added for reconstruction of paving brackets.
 - b. Circle note 12 was revised.
 - c. Signatures were updated.
- 2) On Sheet 2R of 22 for Bridge No. 7268 the following changes were made:
 - a. In the "Schedule of Quantities", the quantity for Anchorages Type Reinf Bars changed from 92 each to 176 each.
 - b. In the "Schedule of Quantities", a pay item was added for Structural Steel (3306) with a quantity of 1400 pounds.
 - c. In the "Schedule of Quantities", a pay item was added for Furnish and Install Beam Stiffeners with a quantity of 8 each.
- 3) On Sheet 6R of 22 for Bridge No. 7268 the following changes were made:
 - a. A second reinforcing anchorage was added to the paving bracket, with quantities adjusted accordingly.
 - b. A note was added specifying the paving bracket is level.
 - c. A call-out for the deck protection plate was added.
- 4) On Sheet 8R of 22 for Bridge No. 7268 the following changes were made:
 - a. Added beam stiffener angles and call-outs to "Framing Plan".
 - b. Added circle note S1 for beam stiffener angles.
- 5) On Sheet 9R of 22 for Bridge No. 7268 the following changes were made:
 - a. Added beam stiffener angle details.

- 6) On Sheet 12R of 22 for Bridge No. 7268 the following changes were made:
 - a. Added beam stiffeners to "Summary of Quantities".
 - b. Added beam stiffener to "Section B-B".
- 7) On Sheet 19R of 22 for Bridge No. 7268 the following changes were made:
 - a. Revised details for F1 bearings.

Plans for Bridge No. 27504:

- 8) On Sheet 1R of 34 for Bridge No. 27504 the following changes were made:
 - a. Notes and call-outs were added for reconstruction of paving brackets.
 - b. Circle note 12 was revised
 - c. Signatures were updated.
- 9) On Sheet 2R of 34 for Bridge No. 27504 the following changes were made:
 - a. In the "Schedule of Quantities", the quantity for Structural Steel (3309) changed from 2000 pounds to 3400 pounds; and the quantity for Anchorages Type Reinf Bars changed from 96 each to 184 each.
 - b. In the "Schedule of Quantities", a pay item was added for Furnish and Install Beam Stiffeners with a quantity of 8 each.
- 10) On Sheet 3R of 34 for Bridge No. 27504 the following changes were made:
 - a. Deck thickness was revised in "Reconstructed Transverse Section".
- 11) On Sheet 5R of 34 for Bridge No. 27504 the following changes were made:
 - a. Circle note 3 was revised.
- 12) On Sheet 6R of 34 for Bridge No. 27504 the following changes were made:
 - a. A second reinforcing anchorage was added to the paving bracket, with quantities adjusted accordingly.
 - b. A note was added specifying the paving bracket is level.
 - c. A call-out for the deck protection plate was added

- 13) On Sheet 8R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised notes to clarify new steel and salvaged steel.
 - b. Added stiffener angles and call-outs to "Framing Plan".
 - c. Added circle note S1 for stiffener angles.
- 14) On Sheet 9R of 34 for Bridge No. 27504 the following changes were made:
 - a. Added deflection ordinate at field splice location.
 - b. Added notes to clarify new and salvaged steel.
 - c. Added stiffener angle to "Beam 7 Elevation".
- 15) On Sheet 11R of 34 for Bridge No. 27504 the following changes were made:
 - a. Added beam stiffener angle details.
- 16) On Sheet 12R of 34 for Bridge No. 27504 the following changes were made:
 - a. Added quantity for beam stiffeners in "Summary of Quantities".
 - b. Revised circle note 12.
 - c. Added circle note 13.
 - d. Clarified 1'-2" dimension in "Partial Deck Plan".
- 17) On Sheet 13R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised circle note 12.
 - b. Added circle note 13.
 - c. Clarified 1'-2" dimension in "Partial Deck Plan".
- 18) On Sheet 14R of 34 for Bridge No. 27504 the following changes were made:
 - a. Added descriptions to Panel P1 and P3 titles.
- 19) On Sheet 16R of 34 for Bridge No. 27504 the following changes were made:

- a. Revised haunch forming scheme and call-outs.
- b. Revised note for reinforcing bars attached to vertical adjustment device.
- 20) On Sheet 18R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised required grout strength in Sequence note 3.
 - b. Switched order of Sequence steps 5 and 6, and revised note 6.
 - c. Revised Sequence notes 7, 9, and 10.
- 21) On Sheet 21R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised haunch forming scheme.
- 22) On Sheet 22R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised haunch forming scheme.
- 23) On Sheet 24R of 34 for Bridge No. 27504 the following changes were made:
 - a. Added longitudinal section at pier.
- 24) On Sheet 26R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised rail rustication from $\frac{3}{4}$ " to $\frac{1}{2}$ ".
- 25) On Sheet 31R of 34 for Bridge No. 27504 the following changes were made:
 - a. Revised details for F1 bearings

Special Provisions:

26) For SB-7.5, make the following revisions:

a. Replace the title with the following:

(2401) Structural Concrete: (Contractor Concrete Mix Design, Bridge Nos. 7268 and 27504)".

b. Replace the first sentence of the first paragraph with the following:

For the cast-in-place deck of Bridge No. 7268 and the precast deck panels of Bridge No. 27504, the Contractor shall design a 3Y33HP concrete mix that will minimize cracking.

- c. Delete SB-7.5.C.7 (Modulus of elasticity tests)
- d. Delete SB-7.5.C.8 (Creep and shrinkage tests)
- e. Revise the title of SB-7.5.E to "Slab Placement and Curing".
- f. Revise the title of SB-7.5.E.1 to "Full-depth slab curing", and replace the second paragraph with the following:

Bridge slab shall have conventional wet curing applied immediately following the finishing machine or air screed. The conventional wet curing shall consist of pre-wetted burlap covered with white plastic sheeting. The burlap shall cover 100% of the deck area with no visible openings, the only exception being that area of the deck which will be located beneath the permanent barrier. The wet curing shall be placed no later than 30 minutes after the finishing machine has completed final strike-off of the concrete surface. If, at any time, the Contractor fails to place the wet curing within the 30 minute time period, and to the satisfaction of the Engineer, the Contractor will be assessed a noncompliance charge of \$500.00 for every 5 minute period or any portion thereof, which the Engineer determines that the Contractor has not complied. The non-compliance charge, set forth above, may be assessed more than once. The slab surface shall be kept continuously wet for an initial curing period of at least 7 days. The Contractor must provide adequate personnel to ensure the burlap is maintained in a wet condition on weekends and/or holidays. In order to comply with the wet curing requirement a work bridge following the finish machine may be required, and an additional center rail may be required on wide bridges.

If for any reason wet burlap cannot be placed within 30 minutes after carpet dragging, apply a membrane curing compound within 30 minutes meeting the requirements of MnDOT Spec. 3754, section B (Requirements for Concrete Pavement Membrane Curing Compound). Apply the curing compound with approved power-operated spray equipment. Place the membrane cure material homogeneously to provide

a uniform solid white opaque coverage on all exposed concrete surfaces (equal to a white sheet of paper). The membrane cure shall be placed within 30 minutes of concrete placement unless otherwise directed by the Engineer. Failure to comply with this provision will result in a price reduction for the concrete item involved in accordance with MnDOT Spec. 1503. The curing compound is not a substitute for the cure specified above and below, but is required for moisture retention until the conventional wet curing material can be placed.

- 27) For SB-7.6C, add the following lab testing requirements:
 - 4. Hardened air content (ASTM C457) at a minimum of 7 days
 - 5. Rapid chloride permeability (ASTM C1202) at 28 days and 56 days (2 specimens for 28 day test and 2 test specimens for 56 day test) (Take 2 specimens form each batch of a 2 batch mix)
 - 6. ASR Expansion results
- 28) For SB-8.1.G, add the following to the end of the paragraph:

Repair of coatings by use of aerosol spray paint is not permitted.

- 29) In the second paragraph under SB-8.4, replace the reference to SB-7.5 with SB-8.5.
- 30) Replace SB-8.7.A.2 with the following:

Immediately before the studs are installed, the weld area shall be cleaned to bright metal by sandblasting or other approved methods.

31) Add the following paragraph after the 6th paragraph of SB-10.A:

The Contractor shall demonstrate the ability to accurately locate and splice post-tensioning ducts by placing two lines of ducts in the mock-up panels, and splicing and sealing the ducts after the placement of the panel concrete. Grouting of these ducts will not be required.

32) Revise the second paragraph under SB-11.1.A as follows:

The concrete for the precast panels shall be mix 3Y33HP, as per SB-7.5.

33) Replace the bulleted note under SB-11.1.B.11 with the following:

Submit written verification acknowledging compatibility with chip seal wearing course. Verification letter shall be signed by representatives from

the chip seal epoxy manufacturer and the grout manufacturer, and indicate acknowledgement from the Contractor.

34) Add the following to the end of SB-11.6.A:

Inplace shear connectors shall be removed such that no more than the attached flange of the angle or channel remains, as long as material left in place does not impede grout flow. Contractor shall take care to not damage beam flanges.

35) Replace SB-11.6.D with the following:

Correct any shifting of precast concrete deck panels during subsequent placement and joining of all the deck panels.

36) Add SB-11.6.E as follows:

Protect openings over girder flanges from surface water and debris intrusion that may occur prior to grouting pockets. Alternatively, Contractor may propose method for cleaning flanges after deck panel placement subject to the approval of the Engineer.

37) Make the following revisions to SB-11.7:

- a. Move Step F (install strands) up to Step D.
- b. Rename Step D (grout transverse joints) as Step E.
- c. Rename Step E as Step F and replace with the following:

Fully tension strands. Do not begin stressing operations until the grout in the transverse joint reaches the strength and age designated on the plans. Stress strands within 36 hours of transverse joint grouting, but not until the panel transverse shear key joint grout has attained the required compressive strength (based on manufacturer's data).

d. Replace Step G with the following:

Grout all ducts (See SB-12) upon approval of stressing logs from the Engineer.

38) Replace SB-11.8.H with the following:

Do not apply distributed loads exceeding 10 psf, or concentrated loads exceeding 1000 pounds, to the precast concrete deck panels until the

structural non-shrink grout in the shear blockouts and the girder haunches has reached a strength of 2500 psi, based on manufacturer's published data.

39) Insert the following after the first sentence of SB-12.3.A:

Storage of prestressing strand in direct contact with the ground will not be permitted.

- 40) Under SB-12.3.E.a, add the following requirement to the grouting operation plan:
 - 13. Warm weather grout contingency plan
- 41) Add the following to the end of the first paragraph of SB-12.3.E.e:

Head boxes are recommended for placement of grout in transverse panel joints and shear blockouts.

42) Revise the second sentence of the second paragraph of SB-13.F.5 to read as follows:

Conduct normal grouting operations at a pressure of 10 psi to 15 psi measured at the grout inlet.

43) Revise the second sentence of the second paragraph of SB-13.F.5 to read as follows:

Concrete for bridge seat repairs shall be either mix 3Y43 or 3Y46

44) Add the following new special provision:

SB-13.10 Beam Stiffeners

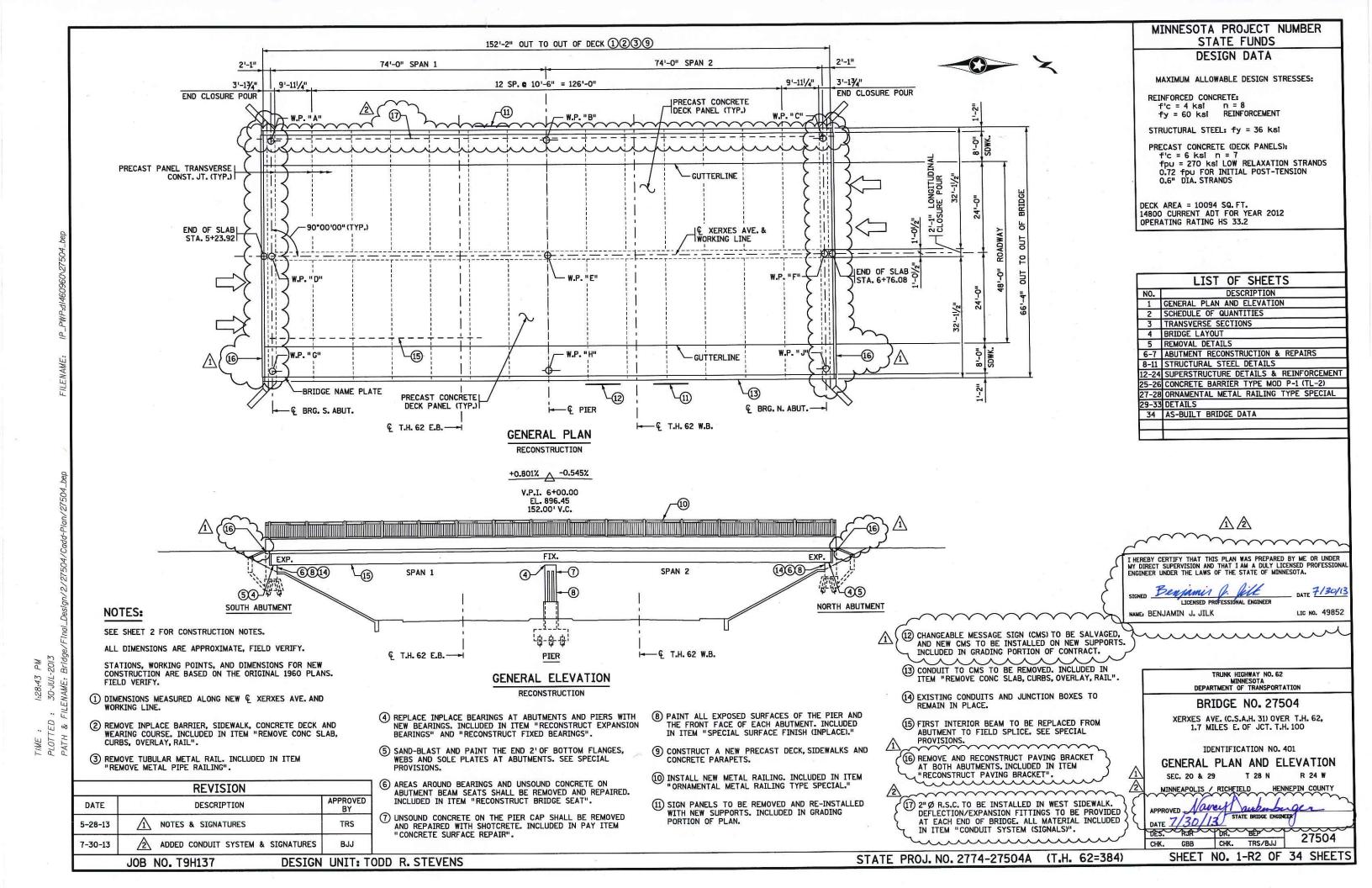
This work consists of furnishing and installing supplemental beam stiffeners at the piers of the inplace beams of Bridge Nos. 7268 and 27504, as shown in the Plans. The work shall be performed in accordance with Mn/DOT 2402, 2471, and the following:

Material for the stiffeners shall conform to the requirements of Mn/DOT 3309, and shall be galvanized after fabrication per Mn/DOT 3394.

Fasteners shall be high strength conforming to the requirements of Mn/DOT 3391, and shall be galvanized per Mn/DOT 3392.

Beam stiffeners will be measured by each stiffener installed and accepted.

Payment for Item No. 2433.602, "FURNISH AND INSTALL BEAM STIFFENERS", will be made at the Contract price per each and shall be compensation in full for all costs of performing the work described above.



	ITEM NO.	ITEM	UNIT	QUANTITY
	2104.501	REMOVE METAL PIPE RAILING	LIN. FT.	304 (P)
Ī	2104.601	REMOVE REGULATED WASTE MATERIAL (BRIDGE)	LUMP SUM	1
	2104.601	HAUL SALVAGED MATERIAL	LUMP SUM	1
Ī	2401.513	TYPE MOD P-1 (TL-2) RAILING CONC (3Y46)	LIN. FT.	305 (P)
	2401.515	SIDEWALK CONCRETE (3Y46)	SQ. FT.	2435 (P)
Ī	2401.541	REINFORCEMENT BARS (EPOXY COATED)	POUND	13820 (P)
Ī	2401.601	STRUCTURE EXCAVATION	LUMP SUM	1
Ī	2401.607	STRUCTURAL CONCRETE (3Y33HP) SPECIAL	CU. YD.	30 (P)
Ì	2401.618	SPECIAL SURFACE FINISH (INPLACE)	SQ. FT.	1500 (P)
	2401.618	BRIDGE DECK PLANING	SQ. FT.	7304 (P)
K	2402.521	STRUCTURAL STEEL (3309)	POUND	(3400) (P)
	2402.533	ERECTING STRUCTURAL METALS	POUND	15200 (P)
Ī	2402.583	ORNAMENTAL METAL RAILING TYPE SPECIAL	LIN. FT.	305 (P)
Ī	2402.601	INSPECT COVER PLATE WELDS	LUMP SUM	1
ı	2402.602	SHEAR STUDS	EACH	2112
ı	2402.602	SPLICE PLATE REPAIR	EACH	1
1	2402.602	WELD REPAIR	EACH	1
Ī	2404.618	CHIP SEAL WEARING COURSE	SQ. FT.	7304 (P)
Ī	2405.601	MOCK-UP PANEL	LUMP SUM	1
ı	2405.616	POST-TENSIONING SYSTEM	SYSTEM	1
ı	2405.618	PRECAST DECK PANEL	SQ. FT.	9375 (P)
ı	2433.505	REMOVE CONCRETE SLAB, CURBS, OVERLAY, RAIL	SQ. FT.	9789
	2433.516	ANCHORAGES TYPE REINF BARS	EACH	(184)
ı	2433.602	RECONSTRUCT EXPANSION BEARINGS	EACH	16
Ī	2433.602	RECONSTRUCT FIXED BEARINGS	EACH	8
	2433.602	EMBEDDED GALVANIC ANODE	EACH	60
(]	2433.602	FURNISH AND INSTALL BEAM STIFFENERS	EACH	8 (P)
Ч	2433.603	RECONSTRUCT PAVING BRACKET	CIN. FT.	130 (P)
	2433.618	CONCRETE SURFACE REPAIR	SQ. FT.	50
1	2433.618	RECONSTRUCT BRIDGE SEAT	SQ. FT.	350
	2476.601	WASTE COLLECTION AND DISPOSAL	LUMP SUM	1
	2478.502	ORGANIC ZINC-RICH PAINT SYSTEM (SHOP)	SQ. FT.	900 (P)
	2478.503	ORGANIC ZINC-RICH PAINT SYSTEM (FIELD)	LUMP SUM	
	2545.509	CONDUIT SYSTEM (SIGNALS)	LUMP SUM	i

CONSTRUCTION NOTES

THE 2005 EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION "STANDARD SPECIFICATIONS FOR CONSTRUCTION" SHALL GOVERN.

PLANS OF INPLACE BRIDGE NO. 27504 ARE AVAILABLE AT THE MINNESOTA DEPARTMENT OF TRANSPORTATION.

THE FIRST TWO DIGITS OF EACH BAR MARK INDICATE THE BAR NUMBER WHICH APPROXIMATES THE NOMINAL DIAMETER OF THE BAR IN MILLIMETERS (mm).

BARS MARKED WITH THE SUFFIX "E" SHALL BE EPOXY COATED IN ACCORDANCE WITH SPEC. 3301.

NO CUTTING WILL BE PERMITTED UNTIL THE CUTTING LIMITS HAVE BEEN OUTLINED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER, REMOVAL AND RECONSTRUCTION SHALL CONFORM TO Mn/DOT SPEC. 2433.

PREFORMED JOINT FILLER MATERIALS ARE INCIDENTAL. PAYMENT TO BE INCLUDED IN PRICE BID FOR OTHER ITEMS.

APPROVED BONDING GROUT TO BE APPLIED TO ALL CONTACT SURFACES BETWEEN NEW AND INPLACE CONCRETE.

THE SUBSURFACE UTILITY INFORMATION IN THIS PLAN IS UTILITY QUALITY LEVEL D. THIS UTILITY QUALITY LEVEL WAS DETERMINED ACCORDING TO THE GUIDELINES OF CI/ASCE 38-02, ENTITLED "STANDARD GUIDELINES FOR THE COLLECTION AND DEPICTION OF EXISTING SUBSURFACE UTILITY DATA".

	REVISION	
DATE	DESCRIPTION	APPROVED BY
5-28-13	1 REVISED QUANTITIES, ADDED PAY ITEMS	TRS
7-30-13	ADDED PAY ITEM	BJJ

CERTIFIED BY Benjamin J. Silk 7/30/13

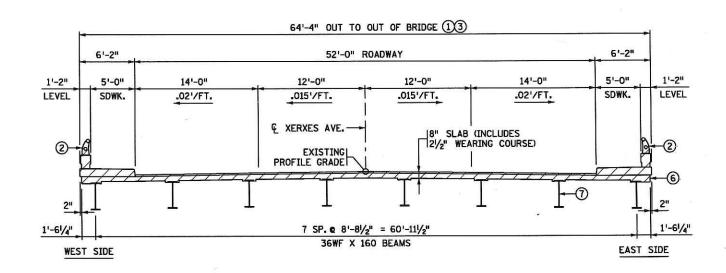
LICENSED PROFESSIONAL ENGINEER

NAME: BENJAMIN J. JILK LIC. NO. 49852

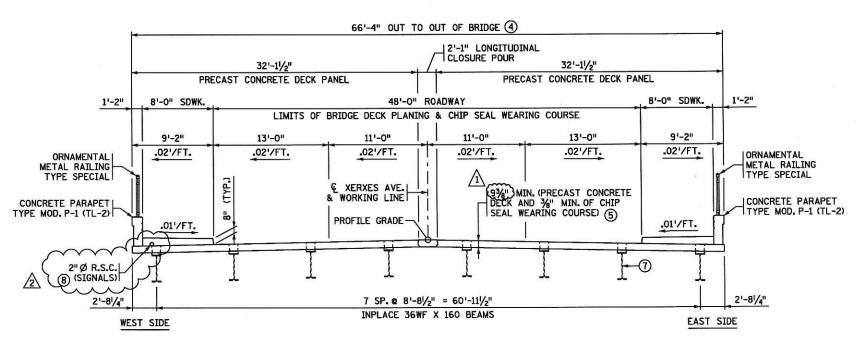
SCHEDULE OF QUANTITIES

| DES: RJR | DR: BEP | APPROVED: 7(3)(3)
| SHEET NO. 2-R2 OF 34 SHEETS

BRIDGE NO. 27504



INPLACE TRANSVERSE SECTION



RECONSTRUCTED TRANSVERSE SECTION

REVISION						
DATE	DESCRIPTION	APPROVED BY				
5-28-13	REVISED DECK PANEL THICKNESS	TRS				
7-30-13	ADDED CONDUIT SYSTEM	BJJ				

CERTIFIED BY Benjamin A. Auf. 7/30/13 LICENSED PROFESSIONAL ENGINEER DATE NAME: BENJAMIN J. JILK LIC. NO. 49852

TRANSVERSE SECTIONS

NOTES:

FIELD VERIFY.

HATCHED AREAS INDICATE REMOVAL.

ALL DIMENSIONS FOR INPLACE STRUCTURE ARE APPROXIMATE,

1 REMOVE INPLACE BARRIER, SIDEWALK, CONCRETE DECK AND WEARING COURSE. INCLUDED IN ITEM "REMOVE CONC SLAB, CURBS, OVERLAY, RAIL"

(2) REMOVE TUBULAR METAL RAIL, INCLUDED IN ITEM "REMOVE METAL PIPE RAILING".

(4) CONSTRUCT A NEW PRECAST DECK, SIDEWALKS, CONCRETE PARAPETS AND ORNAMENTAL METAL RAILINGS.

(5) 93/" MINIMUM TOTAL THICKNESS AFTER BRIDGE DECK PLANING AND CHIP SEAL WEARING COURSE IS APPLIED.

(6) INPLACE CONDUIT TO CMS TO BE REMOVED. INCLUDED IN ITEM "REMOVE CONC SLAB, CURBS, OVERLAY, RAIL".

REPLACE BEAM FROM SOUTH ABUTMENT TO FIELD SPLICE.

8 2"Ø R.S.C. TO BE INSTALLED IN WEST SIDEWALK.
COMBINATION DEFLECTION/EXPANSION FITTING TO BE

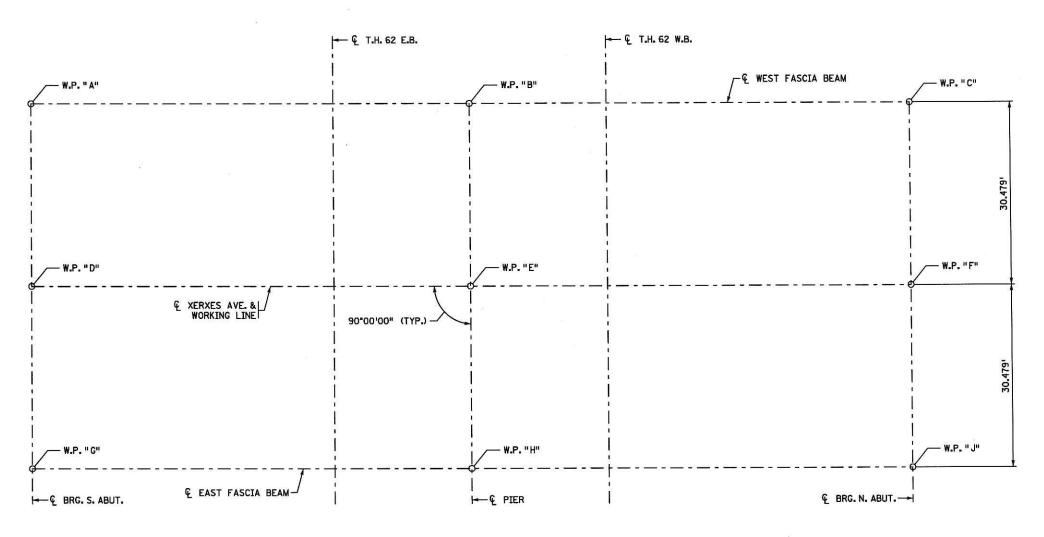
PROVIDED AT EACH END OF BRIDGE. ALL MATERIAL INCLUDED IN ITEM "CONDUIT SYSTEM (SIGNALS)".

(3) CONCRETE END DIAPHRAGMS TO REMAIN INPLACE. REPAIR OF DAMAGE DUE TO DECK REMOVAL OPERATIONS INCLUDED IN ITEM "REMOVE CONC SLAB, CURBS, OVERLAY, RAIL".

DES: RJR DR: BEP APPROVED: 7(30)(3

SHEET NO. 3-R2 OF 34 SHEETS

BRIDGE NO. 27504



WORKING POINT LAYOUT

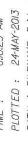
	DIM	ENS]	SNO	B E 7	TWEE	N W	ORKI	NGF	OIN	TS	ELEV.	
POINT	STATION	A	В	С	D	E	F	G	Н	J	TOP OF ROADWAY	POINT
Α	5+26.00		74.00		30.48	80.03			95.87	160.06	895,23	Α
В	6+00.00			74.00		30.48	80.03	95.87		95.87	895.58	В
С	6+74.00						30.48	160.06	95.87		895.43	С
D	5+26.00					74.00		30.48	80.03		895.84	D
E	6+00.00						74.00		30.48	80.03	896.19	E
F	6+74.00									30.48	896.04	F
G	5+26.00								74.00		895.23	G
Н	6+00.00									74.00	895.58	Н
J	6+74.00										895.43	J

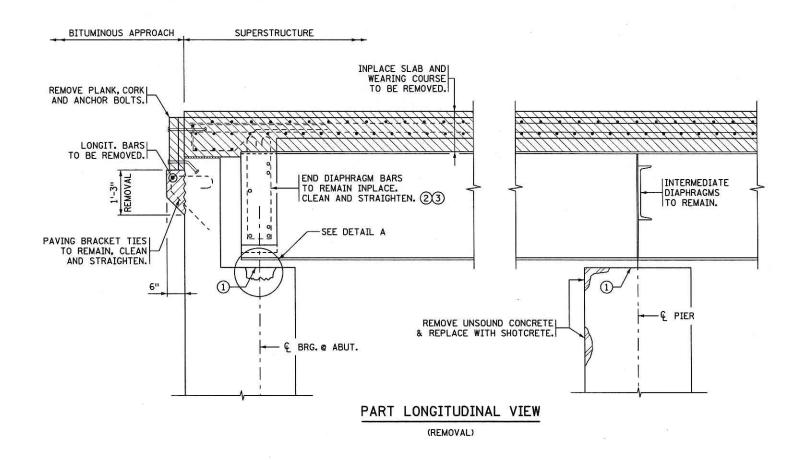
	TOP	OF ROADW	AY TO	BRIDGE	SEAT
		SLAB THICKNESS	STOOL HEIGHT	BEAM HEIGHT	BEARING HEIGHT
	S. ABUT	91/4"	VARIES	36"	25/8"
①	S. ABUT	91/4"	VARIES	357/8"	2%6"
	PIER	91/4"	VARIES	36"	55/8"
	N. ABUT	91/4"	VARIES	36"	25/8"

1 NEW BEAM 7 AT SOUTH ABUTMENT.

CERTIFIED BY	Todd of Stevens	Ela /12	TITLE:		DES:	RJR	DR:	BEP	APPROVED:	BRIDGE NO.
02.11.21.323	LICENSED PROFESSIONAL ENGINEER	DATE	-	BRIDGE LAYOUT	CHKı	GBB	CHK:	TRS	517112	
NAME: TODD R. STEVENS LIC.		C. NO. 21312		BRIDGE LATOUT	SI	HEET	NO. 4	OF 3	4 SHEETS	27504







NOTES:

F.F. DENOTES FRONT FACE B.F. DENOTES BACK FACE

- 1 REPLACEMENT OF BEARING ASSEMBLIES TO BE INCLUDED IN ITEM
 "RECONSTRUCT EXPANSION BEARINGS" & "RECONSTRUCT FIXED BEARINGS".
 INCLUDES REMOVAL OF EMBEDDED PLATES AT ABUTMENT BEARINGS, VOIDS
 NOT FILLED AS PART OF "RECONSTRUCT BRIDGE SEAT" TO BE FILLED WITH AN APPROVED NON-SHRINK GROUT AND FINISHED LEVEL WITH ADJACENT SURFACE.
- 2 GROUTED REINFORCEMENT BARS MAY BE USED TO REPLACE CUT END DIAPHRAGM REINFORCEMENT. GROUTED REINFORCEMENT BARS SHALL BE INCIDENTAL TO ITEM "REMOVE CONCRETE SLAB, CURBS, OVERLAY, RAIL". SEE SPECIAL PROVISIONS.
- 3 REMOVE CONCRETE END DIAPHRAGMS ON EACH SIDE OF BEAM 7 AT THE SOUTH ABUTMENT, REMOVAL INCLUDED IN PAYMENT FOR ITEMS "REMOVE CONCRETE SLAB, CURBS, OVERLAY RAIL PROVIDE MEANS OF TEMPORARY STABILIZATION FOR EXTERIOR GIRDER.

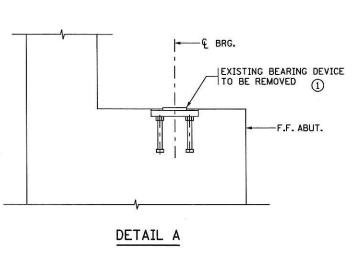
BACK HATCHED AREA DENOTES REMOVAL OF CONCRETE, REINFORCEMENT, RAILING, PLANK AND JOINT MATERIAL. INCLUDED IN PAYMENT FOR ITEM "REMOVE CONCRETE SLAB, CURBS, OVERLAY, RAIL".

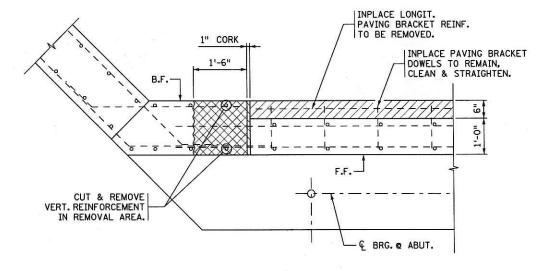
CROSS HATCHED AREA DENOTES REMOVAL OF CONCRETE AND REINFORCEMENT AT WINGWALLS. INCLUDED IN PAYMENT FOR "REMOVE CONCRETE SLAB, CURBS,

DENOTES REMOVAL OF LOOSE CONCRETE ON PIER CAP, INCLUDED IN PAYMENT FOR "CONCRETE SURFACE REPAIR".

HATCHED AREA DENOTES REMOVAL OF PAVING BRACKETS. INCLUDED IN PAYMENT FOR "RECONSTRUCT PAYING BRACKET".

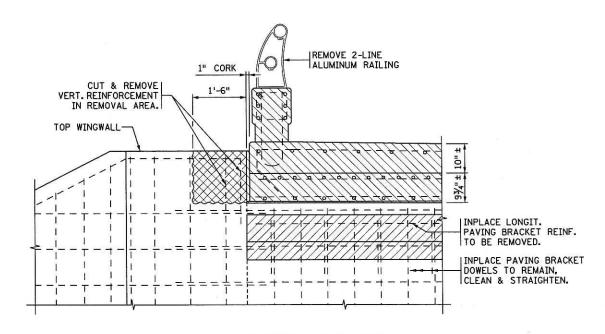
	REVISION	
DATE	DESCRIPTION	APPROVED BY
5-28-13	⚠ NOTES	T.R.S.





REMOVAL PLAN

N.W. & S.E. CORNER SHOWN N.E. & S.W. CORNER SIMILAR



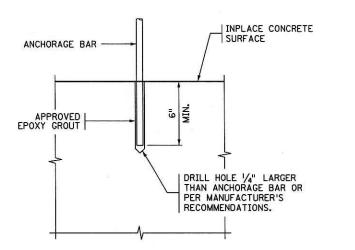
REMOVAL ELEVATION

S.W. & N.E. CORNER SHOWN S.E. & N.W. CORNER SIMILAR

CERTIFIED BY	Jose R.	Stevens	5/29/13	TITLE:
		ROFESSIONAL ENGINEER	DATE	
NAME: TODD R	. STEVENS	LIC.	NO. 21312	

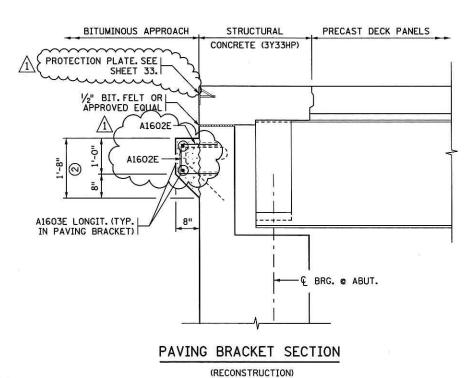
REMOVAL DETAILS

-1	DES:		DF	le	RFL	A	PPROVED:	DOTOGE NO
	CHK:		CH	IK:	TRS		5/29/13	BRIDGE NO.
	SHEET	NC).	5R	OF	34	SHEETS	27504



INCLUDED IN ITEM
"ANCHORAGES TYPE REINF BARS".
SEE THE SPECIAL PROVISIONS FOR
ULTIMATE PULL OUT STRENGTH.

ANCHORAGE DETAIL



NOTES:

F.F. DENOTES FRONT FACE B.F. DENOTES BACK FACE

2) STRUCTURAL CONCRETE (3Y43). INCLUDED IN ITEM "RECONSTRUCT PAVING BRACKET".

	REVISION	
DATE	DESCRIPTION	APPROVED BY
5-28-13	1 PAVING BRACKET REINF. AND NOTES	T.R.S.

BILL OF REINFORCEMENT FOR RECONSTRUCT PAVING BRACKET

BAR	NO.	LENGTH	SHAPE	LOCATION
A1901E A1602E	-8-/	1\1'-6"		BACKWALL DOWEL
A1602E	176	1'-8"		PAVING BRACKET DOWEL
A1603E	18	33'-1"		PAVING BRACKET LONGIT.

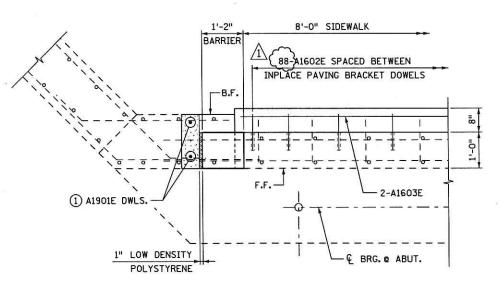
3 PAID FOR AS "ANCHORAGE TYPE REINF BARS". NOT INCLUDED IN WEIGHT OF REINFORCEMENT.

A1602E

SUMMARY OF QUANTITIES FOR RECONSTRUCT TWO PAVING BRACKETS

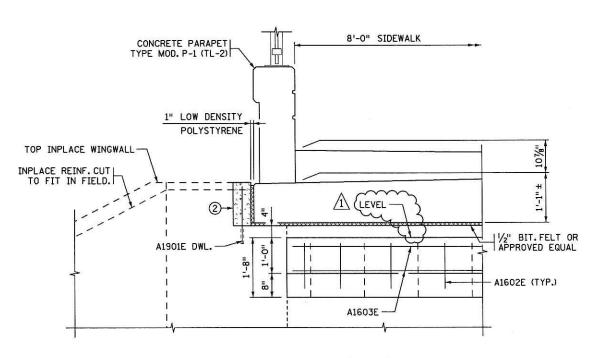
	RECONSTRUCT PAVING BRACKET	130	LIN. FT.
(1)	STRUCTURAL CONCRETE (3Y43)	5	CU. YDS.
$\overline{(1)}$	REINFORCEMENT BARS (EPOXY COATED)	A 280	POUND
_	ANCHORAGE TYPE REINF BAR	/1 184	}EACH
(1)	1/2" BITUMINOUS FELT	133	SQ. FT.
1	1" THICK LOW DENSITY POLYSTYRENE	6	SQ. FT.

(1) INCLUDED IN ITEM "RECONSTRUCT PAVING BRACKET".



PLAN VIEW

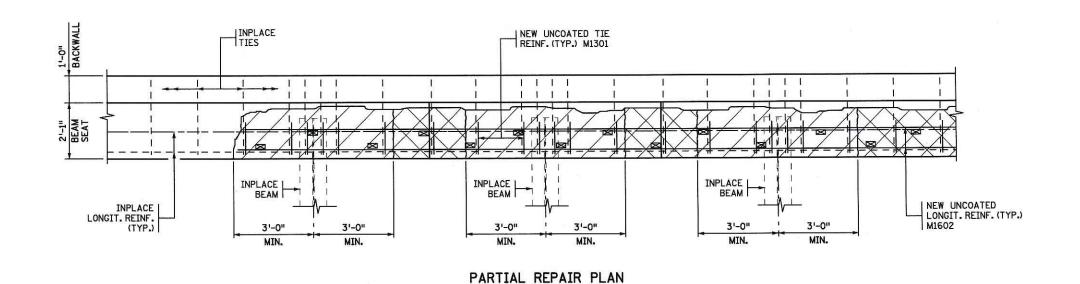
N.W. & S.E. CORNER SHOWN N.E. & S.W. CORNER SIMILAR

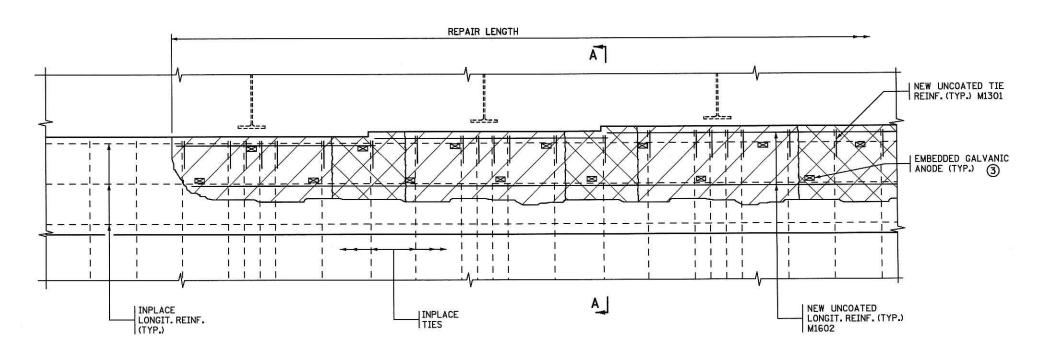


ELEVATION VIEW

S.W. & N.E. CORNER SHOWN S.E. & N.W. CORNER SIMILAR

CERTIFIED BY	Told R Stevens	5/29/12	TITLE: DAVING DRACKET A	DES:	DR:	BEP	APPROVED	DDIDGE NO
	LICENSED PROFESSIONAL ENGINEER	DATE	PAVING BRACKET &	CHK:	CHK:	TRS	5/29/13	BRIDGE NO.
NAME: TODD R	R. STEVENS LIC		BACKWALL RECONSTRUCTION	SHEET NO	O. 6F	R OF 3	34 SHEETS	27504



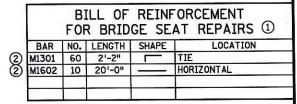


PARTIAL REPAIR ELEVATION

SUMMARY OF QUANTITIES FOR TWO BRIDGE SEAT REPAIRS

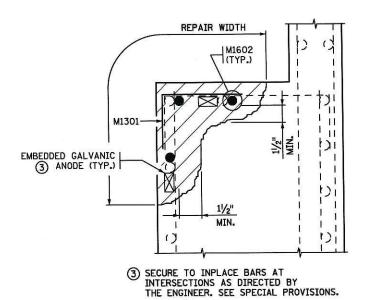
	1011 1110 5112502 02.	
	RECONSTRUCT BRIDGE SEAT	250 SQ. FT.
	CONCRETE SURFACE REPAIR	50 SQ. FT.
	EMBEDDED GALVANIC ANODES	60 EACH
1	REINFORCEMENT BARS	300 POUND
_		

INCLUDED IN ITEM "RECONSTRUCT BRIDGE SEAT"
 SEE SPECIAL PROVISIONS.



② CUT TO LENGTH IN FIELD AS NEEDED.

M1301



SECTION A-A

NOTES:

DENOTES REMOVAL OF UNSOUND CONCRETE AND REPAIR WITH STRUCTURAL CONCRETE (3Y43). INCLUDED IN ITEM "RECONSTRUCT BRIDGE SEAT". DETAILS SHOW EXAMPLES OF POSSIBLE AREAS NEEDING REPAIR. ACTUAL AREAS SHALL BE DETERMINED BY THE ENGINEER IN THE FIELD.

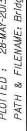
DENOTES REMOVAL OF UNSOUND CONCRETE AND REPAIR WITH SHOTCRETE. INCLUDED IN ITEM "CONCRETE SURFACE REPAIR". DETAILS SHOW EXAMPLES OF POSSIBLE AREAS NEEDING REPAIR. ACTUAL AREAS SHALL BE DETERMINED BY THE ENGINEER IN THE FIFLD.

REMOVE AND REPLACE CORRODED REINFORCING AS DIRECTED BY THE ENGINEER.

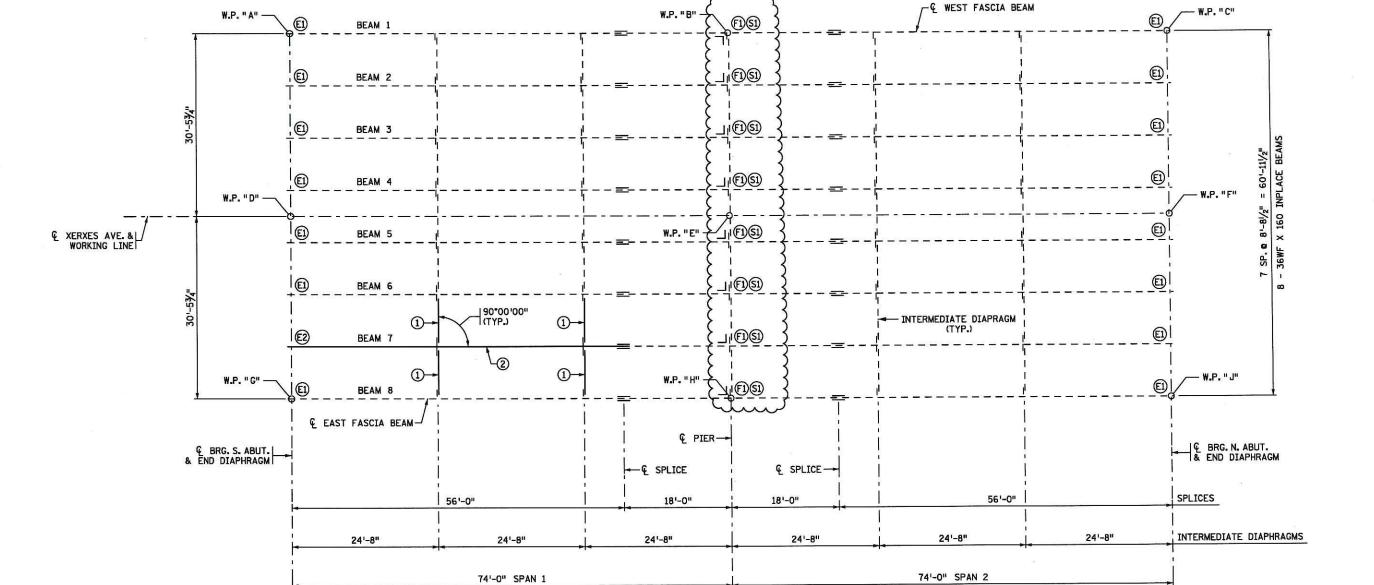
REPAIR WORK MAY NEED TO BE STAGED TO ENSURE STRUCTURAL ADEQUACY. SEE SPECIAL PROVISIONS.



BRIDGE NO. 27504







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STRUCTURAL STEEL NOTES:

ALL STRUCTURAL STEEL SHALL CONFORM TO SPEC. 3309 UNLESS OTHERWISE NOTED.

SHEAR STUD CONNECTORS TO BE INCLUDED IN WEIGHT OF "FURNISHING STRUCTURAL STEEL (3309)" AND CONFORM TO SPEC. 3391.

SPECIAL ASSEMBLY PER MN/DOT SPEC. 2471.3H1 WILL BE REQUIRED FOR BEAM SPLICES.

ALL BOLTED CONNECTIONS SHALL BE MADE WITH $\frac{7}{8}$ " DIAMETER A325 BOLTS, EXCEPT

SHEAR CONNECTORS TO PROJECT A MINIMUM OF 2" INTO STRUCTURAL SLAB. IN NO CASE SHALL THEY BE INSTALLED CLOSER THAN 1" FROM TOP OF STRUCTURAL SLAB. ENGINEER TO FIELD VERIFY BEAM ELEVATION AND AUTHORIZE STUD LENGTH.

ALL NEW AND REPLACEMENT STRUCTURAL STEEL AND EXISTING BEAM ENDS TO BE PAINTED.

SEE SPECIAL PROVISIONS.

REVISION							
DATE	DESCRIPTION	APPROVED BY					
5-28-13	ANGLE STIFFENERS AT PIER, NOTES	TRS					

FRAMING PLAN

NOTES:

- (E1) DENOTES NEW EXPANSION BEARING ASSEMBLY TYPE 1.
- E2 DENOTES NEW EXPANSION BEARING ASSEMBLY TYPE 2.
- (F1) DENOTES NEW FIXED BEARING ASSEMBLY TYPE 1.

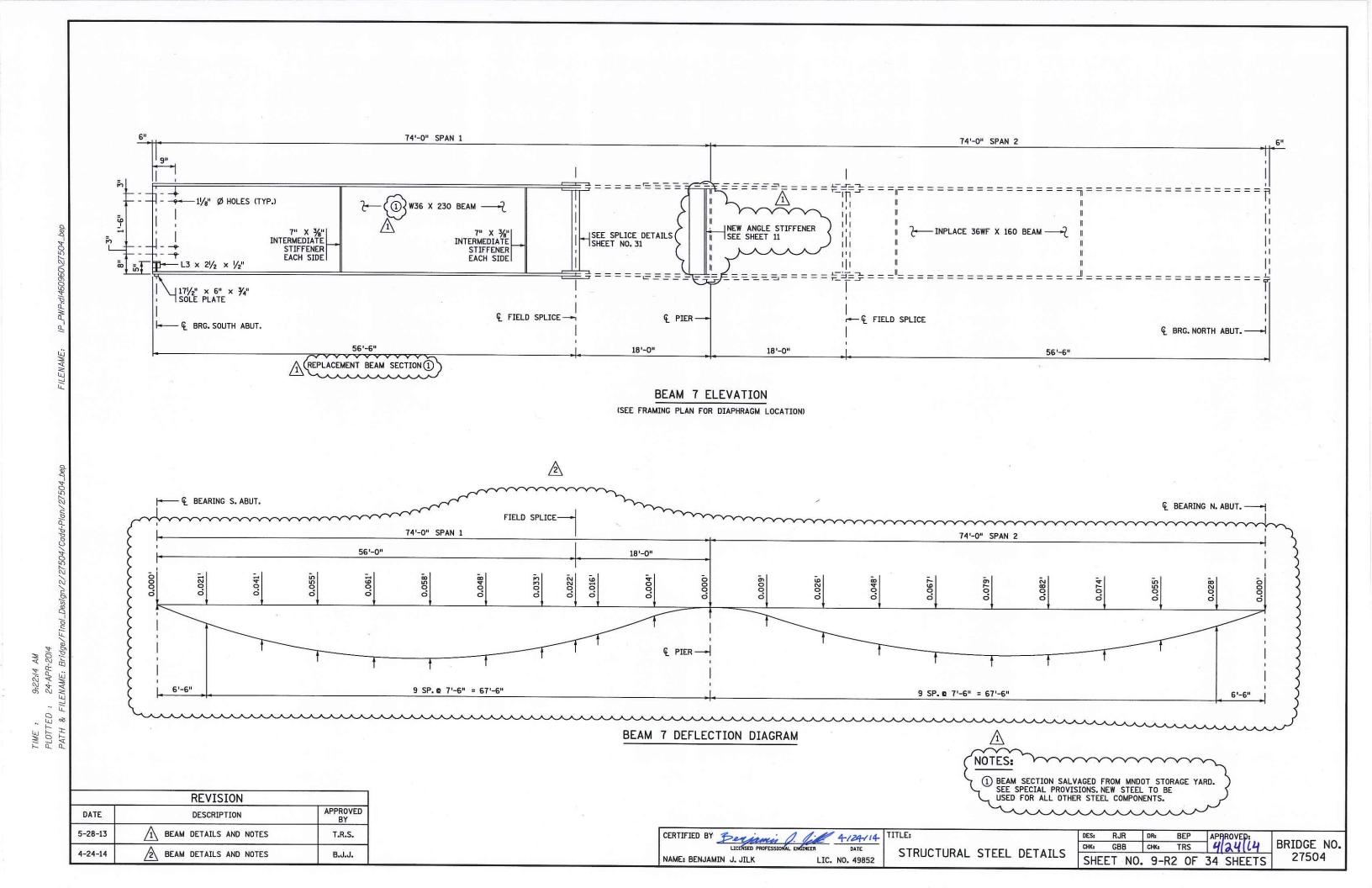
- (F) DENOTES NEW FIRED BEARING ASSEMBLY TIPE I.

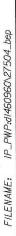
 (S) DENOTES NEW ANGLE STIFFENERS AT PIER BEARINGS.

 (1) DENOTES NEW 5/6" BENT P INTERMEDIATE DIAPRAGMS WITH NEW CONNECTION STIFFENERS.

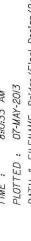
 (2) DENOTES REPLACEMENT W36 X 230 BEAM SECTION. BEAM TO BE HEAT-CAMBERED TO MEET ANTICIPATED DEFLECTION.

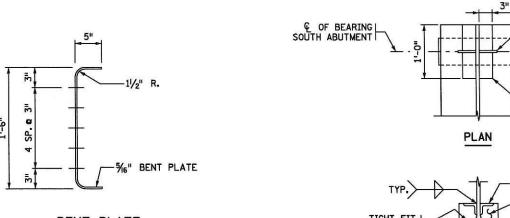
CERTIFIED BY	Todd R Stevens	5/29/13	TITLE:	G.		DESa CHKs	RJR GBB	DR:	BEP TRS	APPROVED:	BRIDGE NO.
NAME: TODD F	LICENSED PROFESSIONAL ENGINEER R. STEVENS	DATE LIC. NO. 21312	STRUCTURAL	STEEL	DETAILS	SH		NO. 8R	1000	34 SHEETS	27504



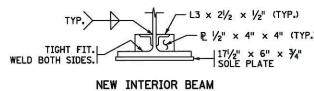








BENT PLATE INTERMEDIATE DIAPHRAGM

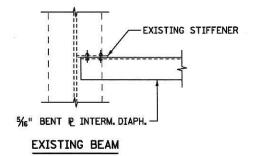


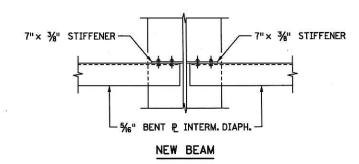
AT ABUTMENT

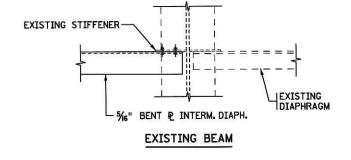
P 1/2" × 4" × 4" (TYP.)

| 171/2" × 6" × 3/4" | SOLE PLATE

L3 x 21/2 x 1/2" (TYP.)





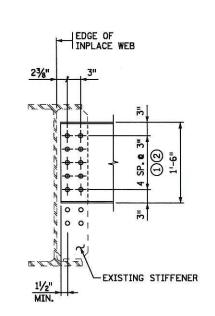


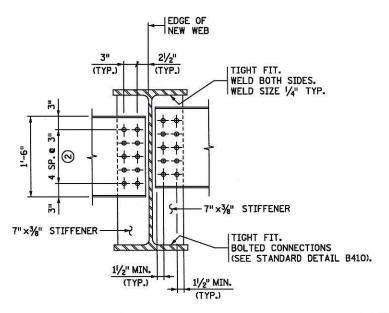
NOTES:

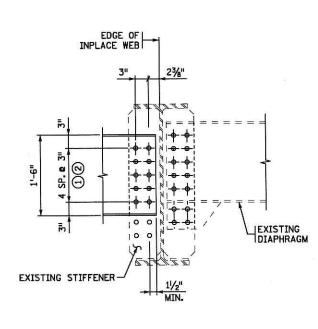
ALL STEEL SHALL CONFORM TO Mn/DOT SPEC. 3309.

① DIMENSIONS TAKEN FROM THE ORIGINAL 1962 SHOP DRAWINGS. FIELD DRILL NEW BENT PLATE INTERMEDIATE DIAPRAGMS TO EXISTING STIFFENERS.

2) FIELD DRILL 15/6" Ø HOLES FOR 1/8" Ø A325 BOLTS (TYP.)







EXISTING FASCIA BEAM BEAM 8

ELEVATION

PLAN

NEW INTERIOR BEAM BEAM 7

ELEVATION

PLAN

EXISTING INTERIOR BEAM BEAM 6

INTERMEDIATE DIAPHRAGM

(CONNECTION TO EXISTING BEAM)

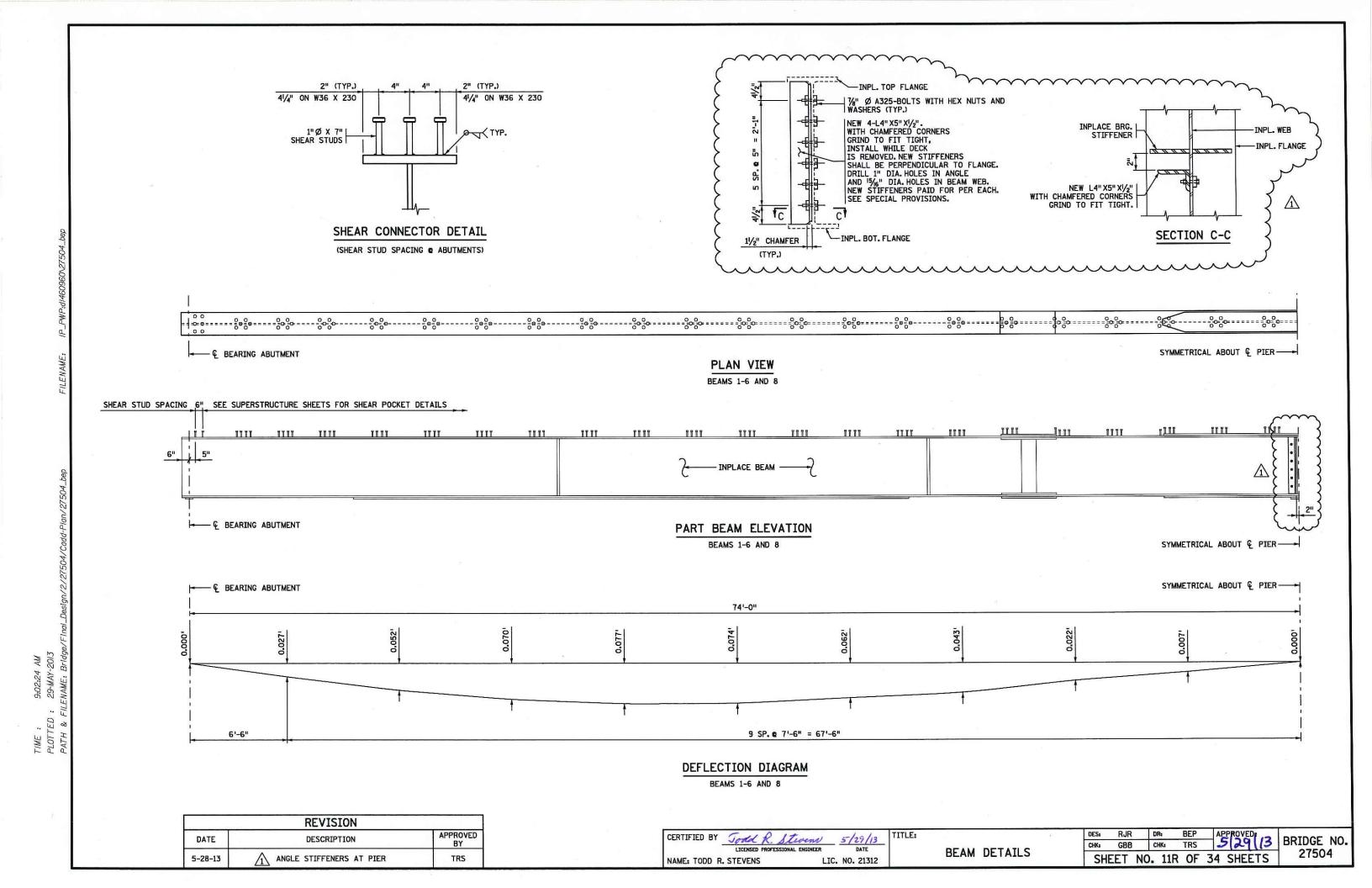
INTERMEDIATE DIAPHRAGM

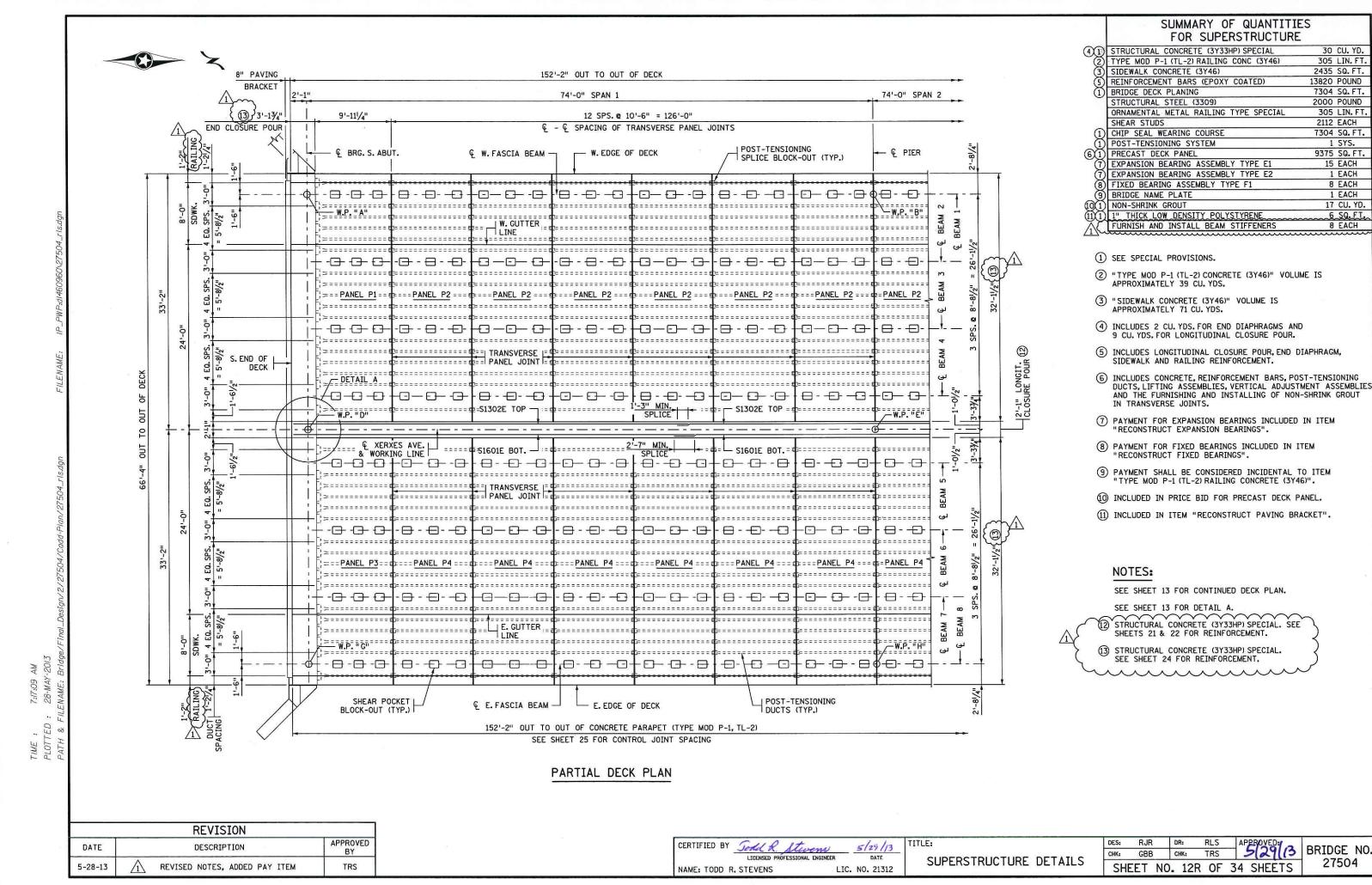
(CONNECTION TO EXISTING BEAM)

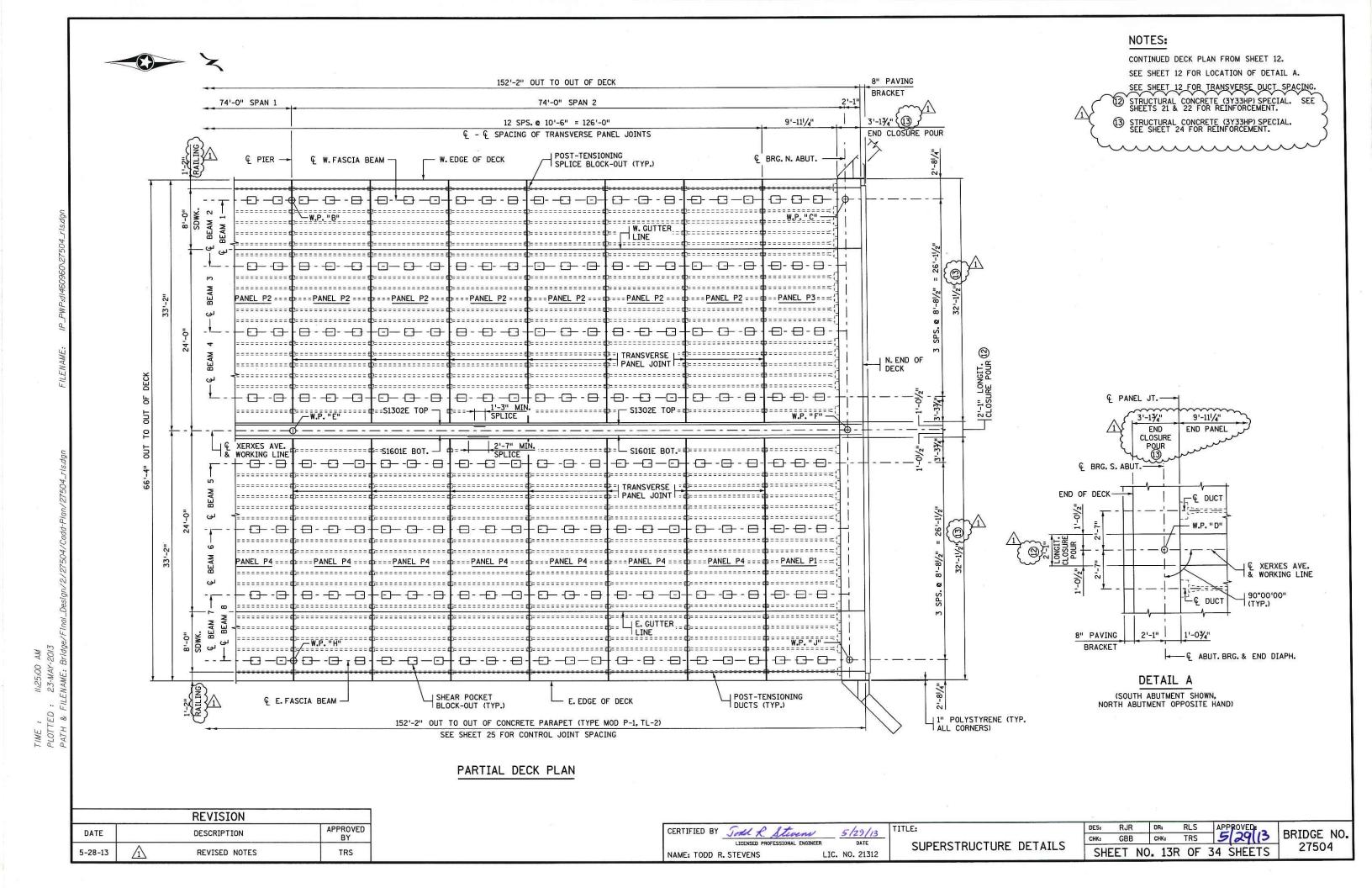
CERTIFIED BY	Godd R. St.	veni	5/7/13	יו
	LICENSED PROFESSION	AL ENGINEER	DATE	
NAME. TODO E	STEVENS	LTC.	NO. 21312	

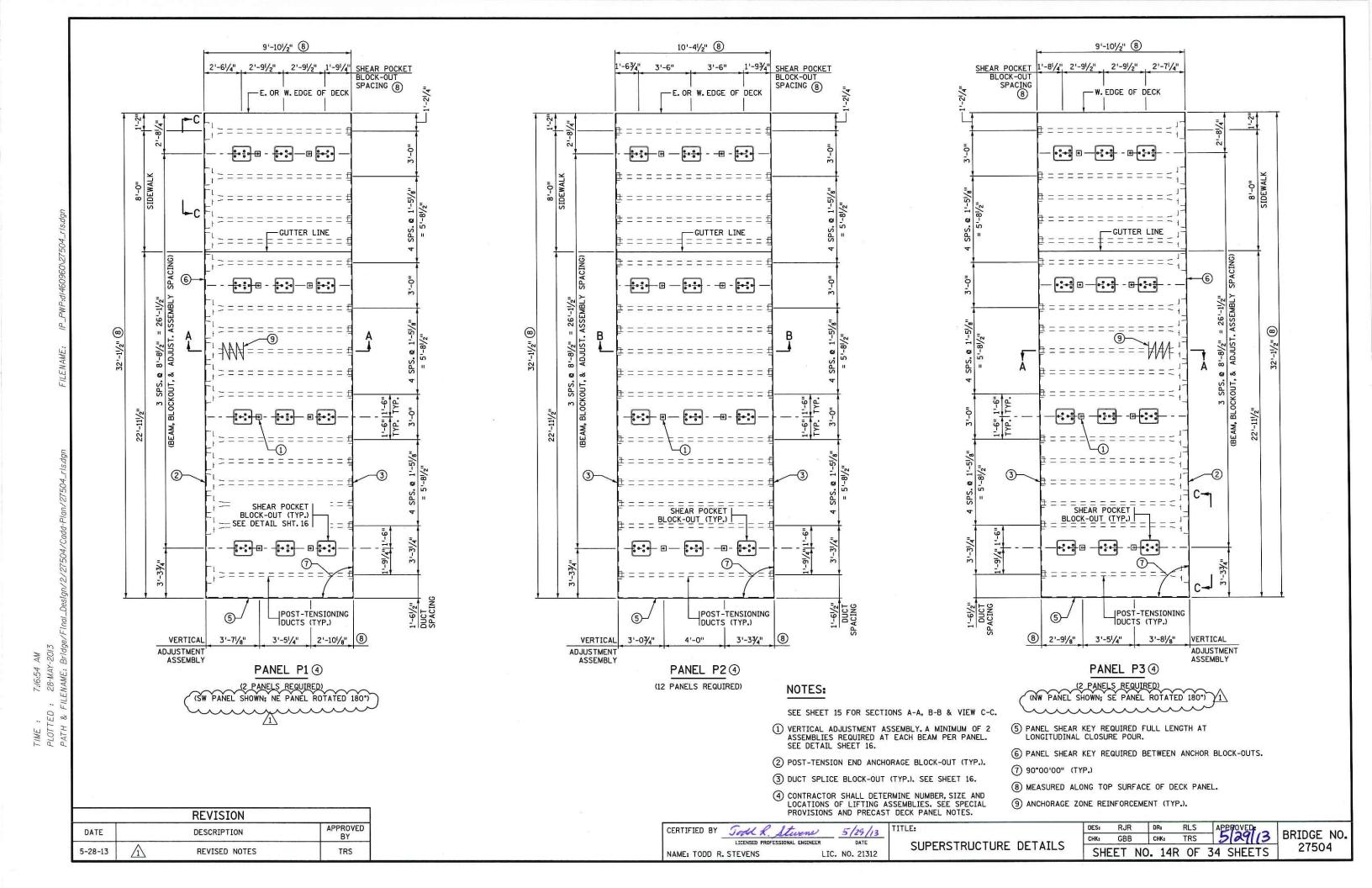
TITLE: STRUCTURAL STEEL DETAILS

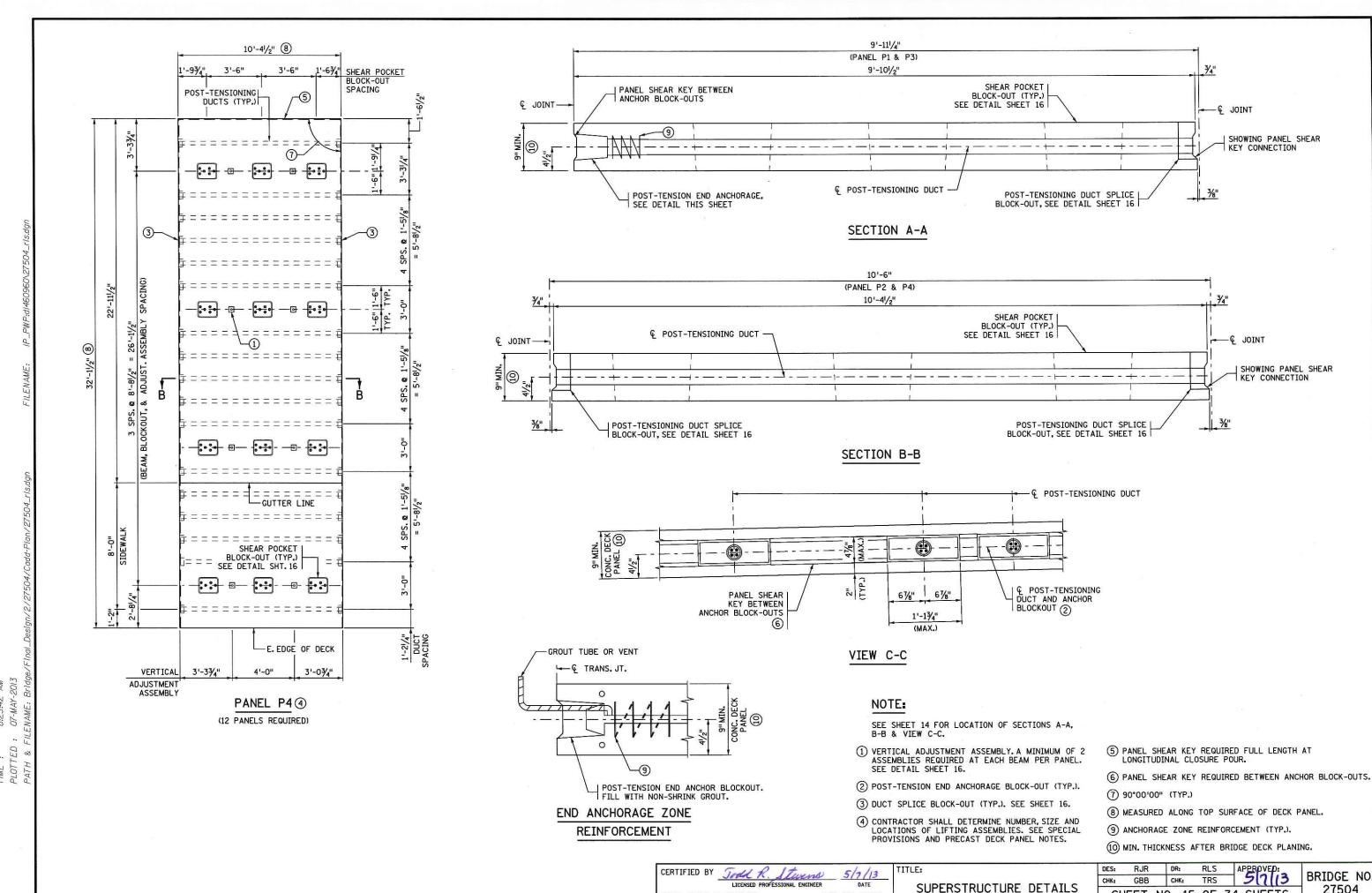
APPROVED: 5(7(13 RJR BEP BRIDGE NO. CHKs GBB CHKs TRS 27504 SHEET NO. 10 OF 34 SHEETS











NAME: TODD R. STEVENS

LIC. NO. 21312

TRS APPROVED: BRIDGE NO. 27504 SHEET NO. 15 OF 34 SHEETS

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—ۅ JOINT

SHOWING PANEL SHEAR

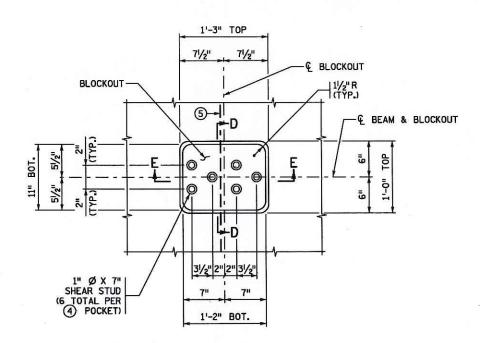
KEY CONNECTION

SHOWING PANEL SHEAR KEY CONNECTION

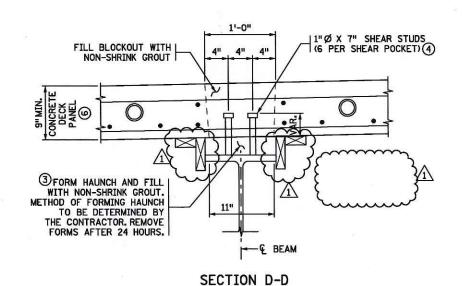


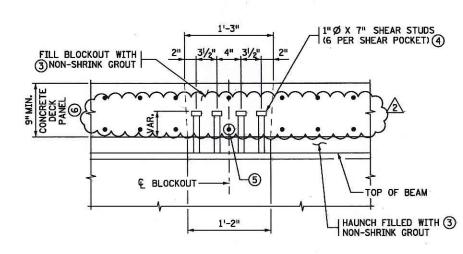




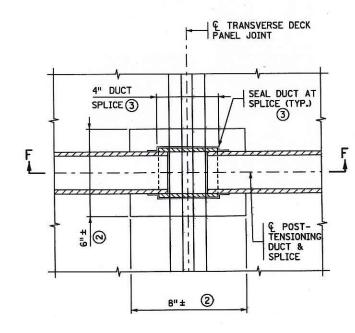


SHEAR POCKET BLOCKOUT DETAIL

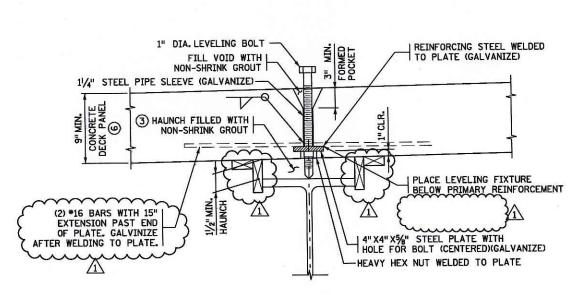




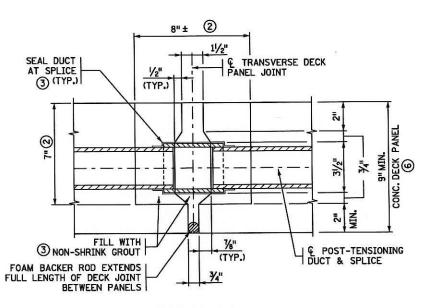
SECTION E-E



PLAN VIEW AT DUCT CONNECTION



VERTICAL ADJUSTMENT DETAIL 1

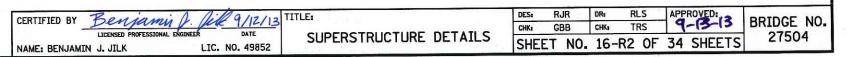


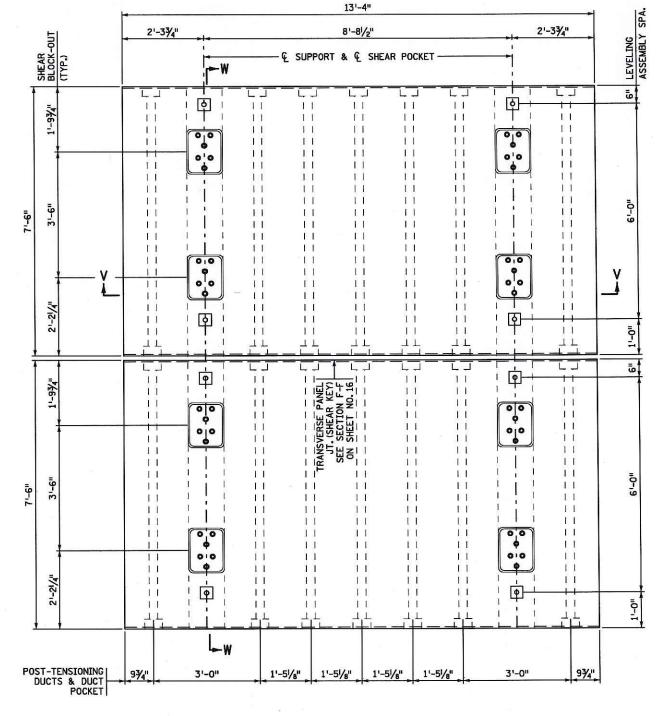
SECTION F-F
(PANEL SHEAR KEY)

REVISION							
DATE	DESCRIPTION	APPROVED BY					
5-28-13	1 REVISED ADJUSTMENT & HAUNCH DETAILS	TRS					
9-12-13	REVISED SECTION E-E NO LONG. BARS THRU SHEAR POCKET	BJJ					

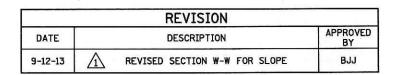
NOTES:

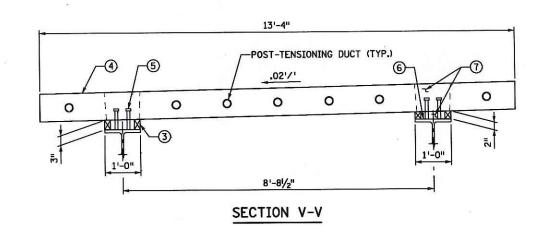
- ① YERTICAL ADJUSTMENT ASSEMBLY. A MINIMUM OF 2 ASSEMBLIES REQUIRED AT EACH BEAM PER PANEL. SEE SPECIAL PROVISIONS.
- 2) POST-TENSIONING DUCT SPLICE BLOCK-OUT (TYP.).
- 3 SEE SPECIAL PROVISIONS.
- (4) CONTRACTOR SHALL PROVIDE GROUNDING METHOD PRIOR TO FIELD WELDING. THE FURNISHING AND INSTALLATION OF THE SHEAR STUDS SHALL BE INCLUDED IN THE PRICE BID FOR "SHEAR STUDS".
- (5) TRANVERSE REINF. BAR THROUGH BLOCKOUT.
- (6) MIN. THICKNESS AFTER BRIDGE DECK PLANING.

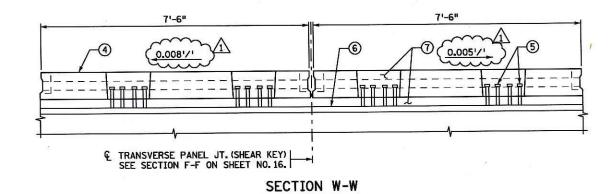




MOCK-UP PANEL DETAIL







MOCK-UP PANEL NOTES:

SEE SHEET 16 FOR ADDITIONAL DETAILS FOR THE MOCK-UP PANEL. CONTRACTOR SHALL PROVIDE AT LEAST FOUR LIFTING ASSEMBLIES PER MOCK-UP PANEL IDENTICAL TO THE LIFTING ASSEMBLIES FOR THE PRECAST CONCRETE DECK PANELS.

REINFORCEMENT SHALL CONSIST OF TWO MATS WITH BAR SIZE AND SPACING SIMILAR TO THAT OF THE PRECAST CONCRETE DECK PANELS SHOWN ON SHEETS 19 & 20.

THE SAME SIZE POST-TENSIONING DUCT, THE SAME PROPOSED VERTICAL ADJUSTMENT ASSEMBLY, THE SAME SIZE SHEAR BLOCKOUT AND DUCT SPLICE POCKET SHALL BE USED AS SHOWN FOR THE PRECAST CONCRETE DECK PANEL. SEE SPECIAL PROVISIONS.

3 USE PROPOSED HAUNCH FORMING MATERIAL.

- PRECAST CONCRETE MOCK-UP PANEL SHALL BE CAST IN THE SAME FORMS AS THE PRECAST DECK PANELS FOR BR. NO. 27504. THE CONCRETE MIX, PANEL THICKNESS, REINFORCEMENT, POCKET DIMENSIONS, TRANSVERSE PANEL JOINT, LIFTING ASSEMBLIES, POST-TENSIONING DUCTS AND LEVELING ASSEMBLIES SHALL BE THE SAME AS USED FOR THE PRECAST DECK PANELS.
- (5) USE (6) 1" Ø SHEAR STUDS PER SHEAR POCKET.
- 6 SUPPORT SURFACE SHALL BE SIMILAR TO TOP OF EXISTING STEEL BEAMS IN WIDTH AND THICKNESS.
- \bigcirc USE PROPOSED NON-SHRINK GROUT IN POCKETS, TRANSVERSE JOINT AND HAUNCH.

CERTIFIED BY Benjamin A Gill	@ 112/12	TITLE:	DES:	RJR	DRs	RLS	APPROVED:	DOTOGE NO
pariforniti y you	9/16/13	CURRENCE DETAILS	CHK	GBB	CHK ₁	TRS	9-13-13	BRIDGE NO.
LICÈNSED PROFESSIONAL ÉNGINEER NAME: BENJAMIN J. JILK LIC	NO. 49852	SUPERSTRUCTURE DETAILS	SHEET NO. 17R OF 34 SHEETS		27504			
MAINT: DEMONISTING OF OTEN	1101 1000		100000000000000000000000000000000000000	THE RESERVE TO 18 YEAR OF THE TO 18 YEAR	PERSONAL PROPERTY	57W 1879-06F		

PRECAST DECK PANEL NOTES:

FABRICATOR SHALL BE RESPONSIBLE FOR EXERCISING CARE IN LIFTING, HANDLING, STORING, AND TRANSPORTATION OF THE PRECAST DECK PANELS TO PREVENT CRACKING OR DAMAGE. PANELS SHALL BE LIFTED BY DEVICES AS DESIGNED BY THE CONTRACTOR AND REVIEWED BY THE ENGINEER.

USE THE PCI DESIGN HANDBOOK, PRECAST AND PRESTRESSED CONCRETE, FIFTH EDITION WITH ALL INTERIMS AND ERRATA FOR THE DESIGN AND DETAIL OF LIFTING SUPPORTS AND HANDLING CONSIDERATIONS (NO CRACKING CRITERIA). LIFTING HARDWARE LEFT INPLACE SHALL BE GALVANIZED AND SHALL HAVE A 3" MIN. CLEAR COVER TO THE TOP OF THE SLAB AND A 1" MIN. CLEAR COVER TO THE BOTTOM OF THE SLAB.

POST-TENSIONING STRANDS SHALL BE UNCOATED, SEVEN-WIRE, LOW-RELAXATION STEEL STRAND OF 0.6" NOMINAL DIAMETER, AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A416, GRADE 270. ALL METHODS EMPLOYED AND PROCEDURES TO BE FOLLOWED IN POST-TENSIONING THE STRANDS SHALL BE SUBJECT TO THE ENGINEER'S APPROVAL.

POST-TENSIONING PARAMETERS:

MAXIMUM JACKING STRESS = 0.8 FU = 216 KSI.
MAXIMUM STRESS AT ANCHOR (SET) = 0.70 FU = 189 KSI.
ASSUMED ANCHOR SET = 0.375 IN.
FOUR STRANDS PER DUCT, JACKING FORCE PER STRAND = 46.8 KIPS.
ASSUMED FRICTION COEFFICIENT = 0.23.
ASSUMED WOBBLE COEFFICIENT = 0.0002/FT.

THE POST-TENSIONING PARAMETERS HAVE BEEN DEVELOPED ASSUMING A FLAT END ANCHORAGE AND ROUND CORRUGATED PLASTIC POST-TENSIONING DUCT.

IF THE PROPOSED POST TENSIONING DOES NOT MEET THESE VALUES, THEN THE CONTRACTOR SHALL ADJUST THE JACKING FORCE TO PRODUCE THE FINAL POST-TENSIONING FORCE LISTED BELOW.

BEGIN STRESSING AT CENTER OF PANELS. DO NOT ALLOW MORE. THAN 12.5% OF THE PRESTRESSING FORCE TO BE ECCENTRIC AT ANY TIME. SUBMIT STRESSING SEQUENCE TO ENGINEER FOR REVIEW PRIOR TO WORK.

CONCRETE IN THE PRECAST DECK PANELS SHALL HAVE A MINIMUM CONCRETE STRENGTH OF 4000 PSI BEFORE REMOVAL FROM CASTING BEDS.

DECK PANELS MUST BE ALLOWED TO SLIDE ON GIRDERS DURING POST-TENSIONING.

FORCE PER STRAND = 39.6 KIPS (AFTER LOSSES DUE TO FRICTION, ANCHORAGE SET AND ELASTIC SHORTENING).

THE CONTRACTOR IS RESPONSIBLE FOR DESIGN OF ALL POST-TENSIONING ELEMENTS AND ANCHORAGE ZONE REINFORCEMENT (REQUIRED FOR SPLITTING, BURSTING, SPALLING, ETC.) INCLUDING THE LOCAL ZONE (REGION IMMEDIATELY SURROUNDING POST-TENSIONING DEVICES) AND THE GENERAL ZONE. DESIGN SHALL CONFORM WITH AASHTO LRFD SPECIFICATIONS.

SPIRAL REINFORCEMENT ALTERNATE IS SHOWN FOR INFORMATION ONLY.

THE PRECAST CONCRETE DECK PANELS SHALL BE FABRICATED TO PLAN DIMENSIONS WITHIN THE RECOMMENDED TOLERANCES SHOWN.

PRECAST DECK PANELS SHALL BE PRODUCED AND PLACED SUCH THAT THERE IS NO MORE THAN 1/4 IN DIFFERENCE IN ELEVATION BETWEEN THE TOP SURFACES OF ADJACENT PANELS.

USE A LIGHT BROOM FINISH FOR THE TOP SURFACES OF PANELS AND AT ALL JOINT SURFACES. USE A SMOOTH FINISH FOR THE BOTTOM OF THE PANELS.

CONTRACTOR SHALL PROVIDE TEST PANEL MOCK-UPS. SEE PLAN DETAILS AND SPECIAL PROVISIONS.

ALL PANEL JOINTS SHALL BE CLEAN AND CONTAIN NO DIRT, OIL, GREASE OR OTHER LOOSE MATERIAL BEFORE PLACING GROUT OR CONCRETE. WATER BLAST AFTER CLEANING.

	REVISION	
DATE	DESCRIPTION	APPROVED BY
5-28-13	REVISED CONSTRUCTION SEQUENCE NOTES	TRS
9-12-13	2 REVISED CONSTRUCTION SEQUENCE NOTES	BJJ

SUPERSTRUCTURE CONSTRUCTION SEQUENCE

THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW A DETAILED CONSTRUCTION SEQUENCE OF WORK TASKS TO BE PERFORMED BEFORE REMOVAL OF THE EXISTING STRUCTURE. DETAILED WORK TASK SEQUENCE SHALL INCLUDE THE INTENDED METHOD FOR FORMING THE GIRDER HAUNCHES & QUALITY CONTROL CONSTRUCTION METHOD FOR OBTAINING THE PROPER ALIGNMENT AND GRADE FOR THE PRECAST DECK PANELS. THE PLANS HAVE BEEN DEVELOPED ASSUMING THE FOLLOWING CONSTRUCTION SEQUENCE.

REMOVE EXISTING CONCRETE DECK; REMOVE EXISTING SHEAR CONNECTORS; REPLACE SECTION OF BEAM 7.
TOP OF BEAMS SHALL BE CLEAN AND CONTAIN NO DIRT, OIL, GREASE, OR OTHER LOOSE MATERIAL BEFORE
PLACING HAUNCH FORMS, PRECAST DECK PANELS AND NON-SHRINK GROUT.

2. ERECT ALL OF THE PRECAST DECK PANELS AS SHOWN IN THE SUPERSTRUCTURE SHEETS.

BEGIN PLACING PANELS AT CENTERLINE OF PIER AND WORK TOWARDS BOTH ABUTMENTS SIMULTANEOUSLY.
PLACE BOTH P2 AND P4 PANELS AT EACH CROSS SECTION BEFORE MOVING TO THE NEXT PAIR OF PANELS.
CARE SHOULD BE TAKEN TO ENSURE THE PRECAST SLAB PANELS ARE IN TIGHT CONTACT WITH THE BACKER
ROD SEPARATING THEM AND PROPER ALIGNMENTS ACHIEVED. USE LEVELING BOLTS TO ACHIEVE THE REQUIRED
GRADE. TORQUE ALL LEVELING BOLTS ON EACH PANEL TO WITHIN 15 PERCENT OF EACH OTHER TO ENSURE
PROPER DISTRIBUTION OF PANEL WEIGHT TO THE SUPPORTING BEAMS. AT NO TIME WILL CONSTRUCTION
EQUIPMENT BE ALLOWED ON THE PRECAST SLAB PANELS UNTIL CONSTRUCTION OF THE PRECAST SLAB. IS
COMPLETE AND THE HAUNCHES AND KEYWAYS HAVE ACHIEVED A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI
THE CONTRACTOR SHALL ENSURE ALL BOLTS ARE IN CONTACT WITH THE TOP FLANGE BEFORE THE PRECAST
SLAB PANELS ARE RELEASED FROM THE ERECTING CRANE AND THE PRECAST SLAB PANELS ARE SOLEY
SUPPORTED BY ALL THE LEVELING BOLTS.

3. FORM THE GIRDER HAUNCHES. NOTES: ALL PANELS SHALL BE ERECTED AND THE PANELS SHALL BE LONGITUDINALLY POST-TENSIONED AND ACCEPTED BY THE ENGINEER PRIOR TO PLACING CONCRETE FOR HAUNCHES (SEE STEP 12 BELOW).

4. JOIN DUCTS FOR POST-TENSIONING TENDONS AT ALL TRANSVERSE JOINTS.IT IS SUGGESTED THAT THE DUCT SPLICE BE ATTACHED TO THE DUCTS PROTRUDING OUT OF THE PANELS BEFORE THE NEXT SUCCESSIVE PANELS ARE ERECTED.

5. INSTALL THE 0.6" POST-TENSIONING STRANDS THROUGH THE POST-TENSIONING DUCTS AND ANCHORAGE SYSTEMS.

6. CLEAN TRANSVERSE JOINT SURFACES. FILL THE TRANSVERSE JOINTS WITH NON-SHRINK GROUT LEVEL WITH OR 1/8" HIGHER THAN THE TOPS OF THE PRECAST DECK PANELS.

7. BEGINNING AT EITHER END OF THE BRIDGE, TENSION THE STRANDS IN EACH POST-TENSIONING
DUCT TO THE SPECIFIED FORCE AND IN THE SECUENCE SHOWN IN THE REVIEWED SHOP DRAWINGS.
TENSIONING SHALL BE COMPLETED WITHIN 36 HOURS OF GROUTING OF TRANSVERSE JOINTS, BUT ONLY IF
GROUT HAS ACHIEVED A COMPRESSIVE STRENGTH OF 3000 PSI.

8. GROUT POST-TENSIONING DUCTS WITH MANUFACTURER'S RECOMMENDED PRODUCT.

9. CAP AND SEAL ALL GROUT VENT TUBES.

10. INSTALL SHEAR STUDS AND HAMMER TEST PER SPECIFICATIONS. VI

11. FILL ALL SHEAR STUD POCKETS IN THE PRECAST DECK PANELS, DUCT SPLICE POCKETS AND HAUNCHES WITH THE SPECIFIED GROUT MIX. CURE TO SPECIFIED STRENGTH.

12. PLACE END CLOSURE POUR WITH CONCRETE MIX (3Y33HP) SPECIAL.

13. PLACE LONGITUDINAL CLOSURE POUR CONCRETE MIX (3Y33HP) SPECIAL.

14. REMOVE LEVELING BOLTS AND FILL HOLES WITH THE SPECIFIED GROUT MIX.

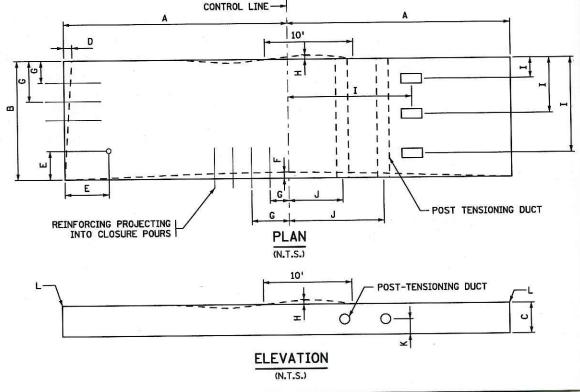
15. PERFORM BRIDGE DECK PLANING. SEE SPECIAL PROVISIONS.

16. PLACE BARRIER RAIL AND SIDEWALKS.

17. SEAL ALL DECK AND JOINT CRACKS WITH JOINT & CRACK SEALANT.
CONTRACTOR TO VERIFY COMPACTABILITY OF ANY JOINT/CRACK SEALANT
WITH CHIP SEAL WEARING COURSE SUPPLIER.

18. PLACE EPOXY CHIP SEAL WEARING COURSE. SEE SPECIAL PROVISIONS.

١	LENGTH MEASURED FROM CONTROL LINE	± 3/6"
3	WIDTH (OVERALL)	± 1/4"
0	DEPTH (OVERALL)	+ 3/16"
)	VARIATION FROM SPECIFIED PLAN END SQUARENESS OR SKEW	± 1/4"
Ε	LOCATION OF LEVELING BOLTS	± 1"
F	SWEEP OVER MEMBER LENGTH:	± 1/4"
G	LOCATION OF PROJECTING REINFORCING MEASURED FROM A COMMON REFERENCE POINT	± 1/2"
Н	LOCAL SMOOTHNESS OF ANY SURFACE	± 1/8" IN 10 FEET
I	LOCATION OF BLOCKOUT FOR SHEAR CONNECTORS	± 1/2"
J	LOCATION OF POST TENSIONING DUCT MEASURED FROM A COMMON REFERENCE POINT	± 1/8"
K	LOCATION OF POST TENSIONING DUCT MEASURED FROM BOTTOM OF PANEL.	± 1/6"
1	ERECTION ELEVATION TOLERANCE	± 1/8"



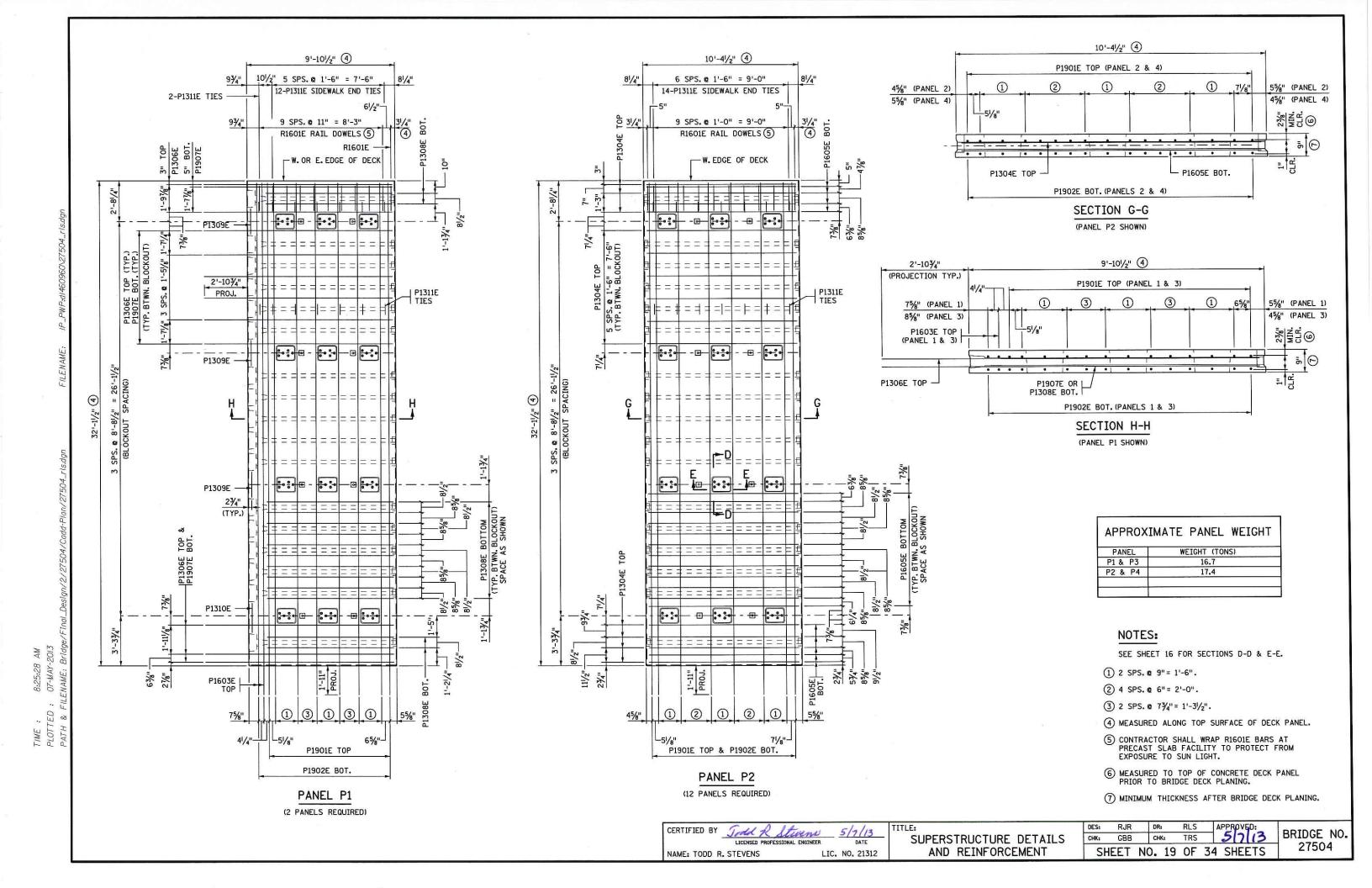


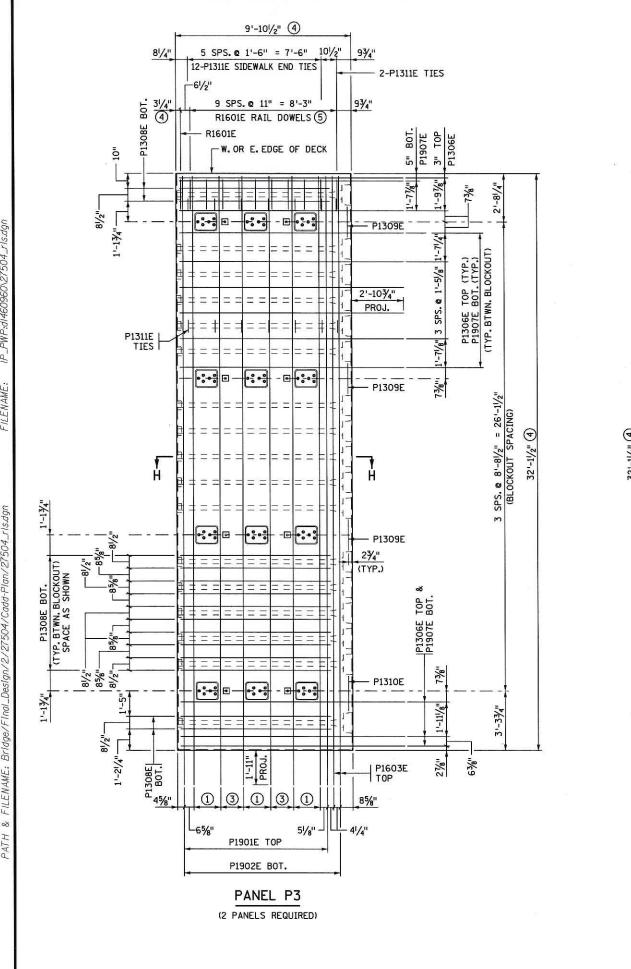
SUPERSTRUCTURE DETAILS

DES: RJR DR: RLS APPROVED:
CHK: GBB CHK: TRS 7-13-13

SHEET NO. 18-R2 OF 34 SHEETS

BRIDGE NO. 27504





. AM -2013

P160 000 -9-- 10-8'-81/2" = 26'-1/2" KOUT SPACING) 32'-1/2" (4) SPS. @ P1311E [-E. EDGE OF DECK 9 SPS. @ 1'-0" = 9'-0" R1601E RAIL DOWELS (5) 6 SPS. @ 1'-6" = 9'-0" 14-P1311E SIDEWALK END TIES

PANEL P4

(12 PANELS REQUIRED)

10'-41/2" (4)

2

1

P1901E TOP & P1902E BOT.

(1)

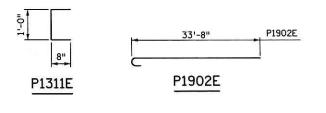
(1)

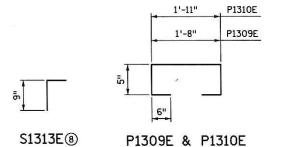
BILL OF REINFORCEMENT FOR PRECAST CONCRETE PANELS 6

BAR NO. LENGTH SHAPE P1901E 460 33'-10" PANELS P1, P2, P3 & P4 TOP PANELS P1, P2, P3 & P4 BOT. P1902E 468 34'-4" P1603E 8 33'-10" ----PANELS P1 & P3 TOP P1304E 600 10'-0" - PANELS P2 & P4 TOP LONGIT P1605E 1080 10'-0" -PANELS P2 & P4 BOT. LONGIT. P1306E 92 | 12'-7" - PANELS P1 & P3 TOP LONGIT. PANELS P1 & P3 BOT. LONGIT. P1907E 92 12'-7" ---PANELS P1 & P3 BOT. LONGIT.

BILL OF REINFORCEMENT FOR SIDEWALK & LONGITUDINAL CLOSURE POUR (7)

	0	, L	DIAGLIC	DINAL	CLOSUNE 1 OUN (
	BAR	NO.	LENGTH	SHAPE	LOCATION
	S1601E	12	52'-4"	·	LONGIT. JT. BOTTOM
	S1302E	12	51'-6"		LONGIT. JT. TOP
	S1311E	48	51'-6"		SIDEWALK LONGITUDINAL
	S1312E	208	7'-7"		SIDEWALK TRANSVERSE
(8)	S1313F	392	1!-7"		SIDEWALK TIE ANCHORAGE (ALT.)





NOTES:

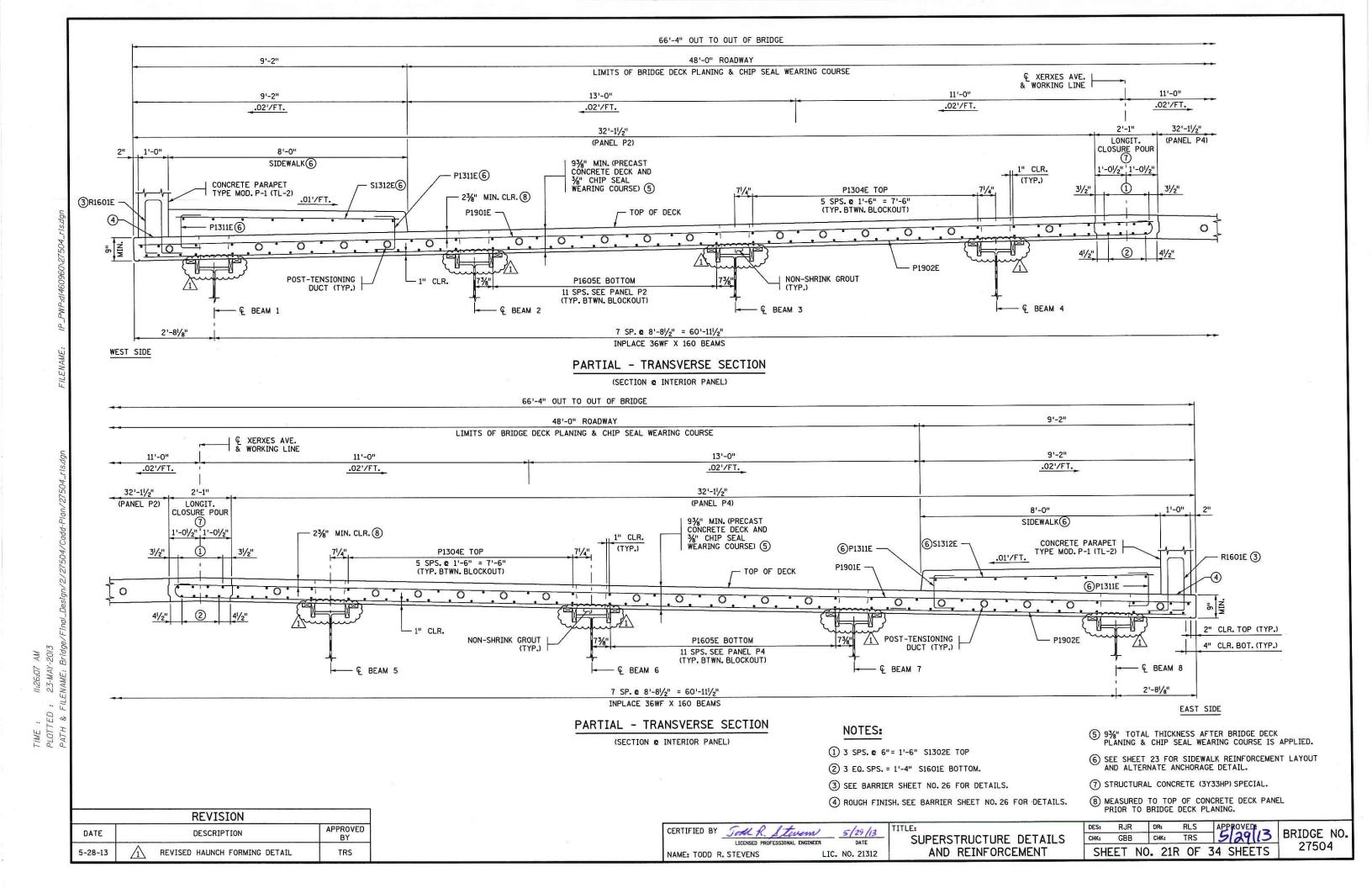
SEE SHEET 16 FOR SECTIONS D-D & E-E. SEE SHEET 19 FOR SECTIONS G-G & H-H.

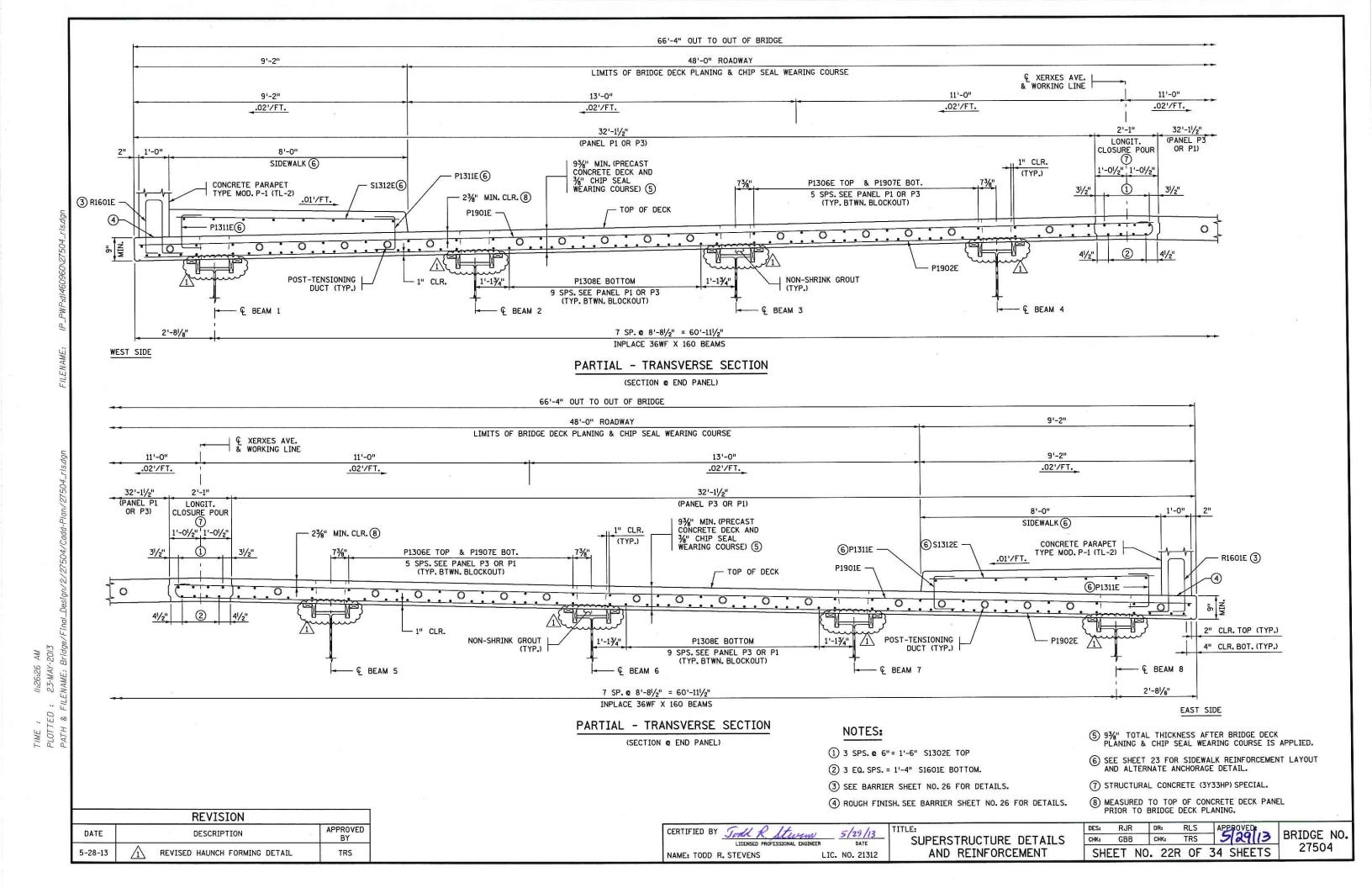
- 1) 2 SPS. @ 9" = 1'-6".
- 2) 4 SPS. @ 6" = 2'-0".
- 3 2 SPS. @ 73/4" = 1'-31/2".
- 4 MEASURED ALONG TOP SURFACE OF DECK PANEL.
- (5) CONTRACTOR SHALL WRAP R1601E BARS AT PRECAST SLAB FACILITY TO PROTECT FROM EXPOSURE TO SUN LIGHT.
- 6 INCLUDED IN PRICE BID FOR "PRECAST DECK PANEL".
- 7 INCLUDED IN PRICE BID FOR "REINFORCEMENT BARS (EPOXY COATED)".
- (8) PAYMENT BASED ON USE OF P1311E. SEE NOTE ON SHEET 23.

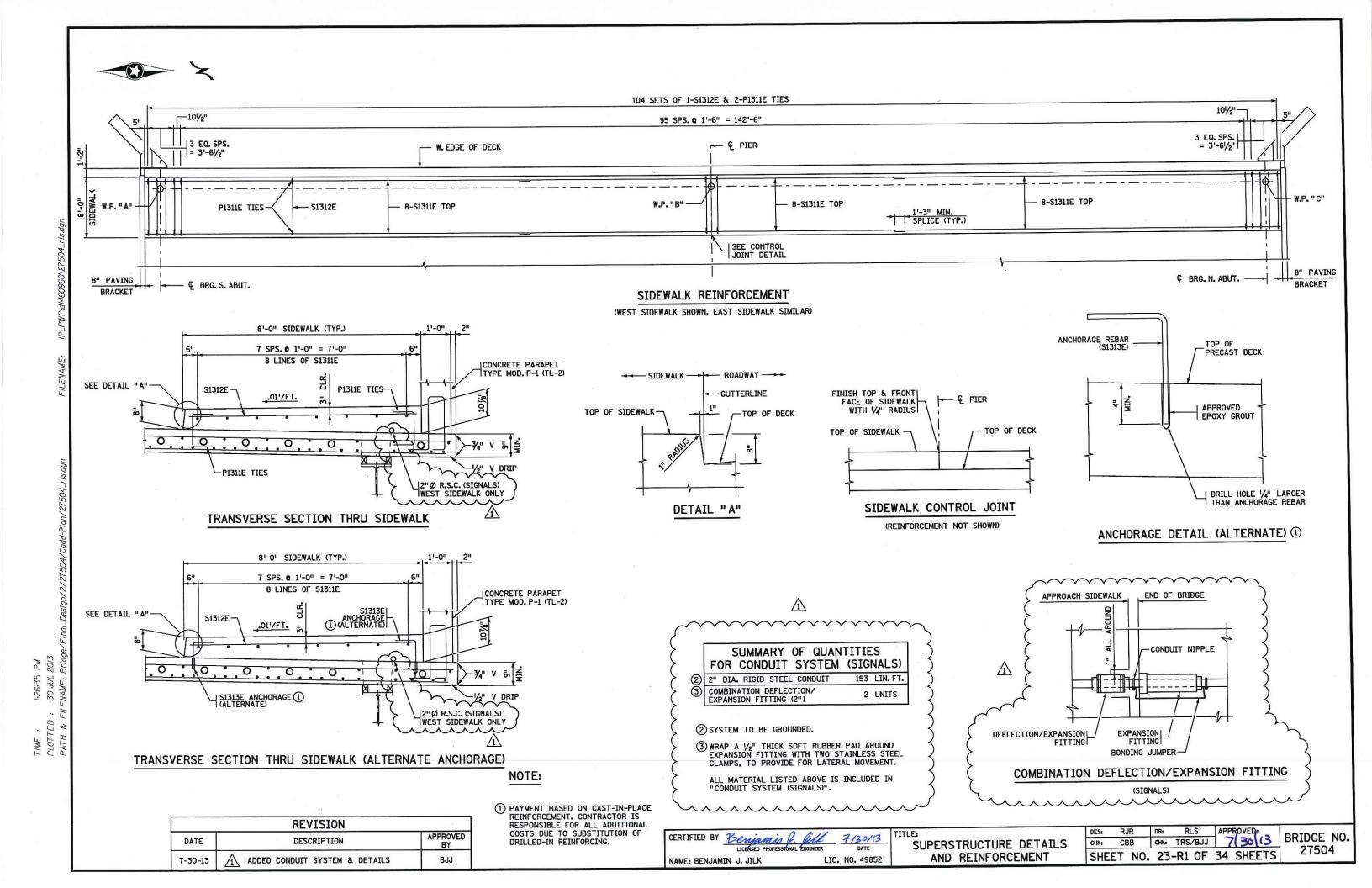
Todd R. Stevens TITLE: CERTIFIED BY 5/7/13 AND REINFORCEMENT LIC. NO. 21312 NAME: TODD R. STEVENS

SUPERSTRUCTURE DETAILS

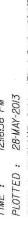
RJR 51713 GBB CHK: TRS CHK₂ SHEET NO. 20 OF 34 SHEETS BRIDGE NO. 27504

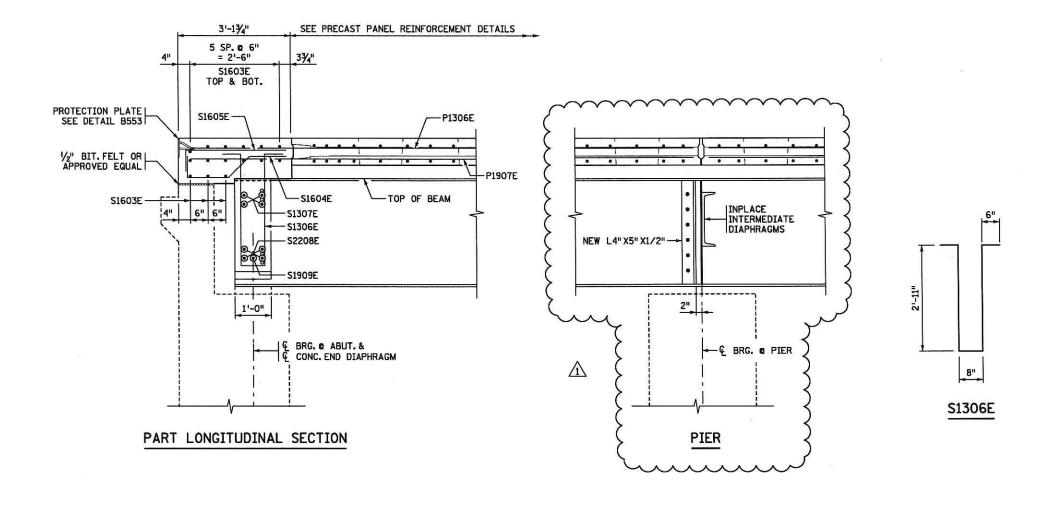


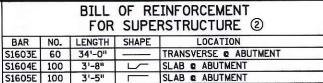






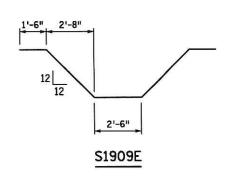


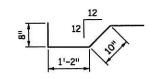




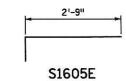
	S1603E	60	34'-0"		TRANSVERSE @ ABUTMENT
1	S1604E	100	3'-8"		SLAB @ ABUTMENT
1	S1605E	100	3'-5"		SLAB @ ABUTMENT
_	S1306E	12	7'-6"	П	END DIAPHRAGM VERTICAL TIE
	S1307E	8	8'-3"		END DIAPHRAGM LONGITUDINAL
	S2208E	8	8'-3"		END DIAPHRAGM LONGITUDINAL
	S1909E	2	13'-0"	~	END DIAPHRAGM LONGITUDINAL
	S2210E	4	5'-0"		END DIAPHRAGM DOWEL THRU BEAM

- SPACED WITH PRECAST PANEL LONGITUDINAL REINFORCEMENT AND CLOSURE POUR LONGITUDINAL REINFORCEMENT.
- ② INCLUDED IN PRICE BID FOR "REINFORCEMENT BARS (EPOXY COATED)".





S1604E



DESa CHK:

3"	5 EQ. SP. = 7'-01/4" 6-S1306E	3" 3" 5 EC	Q. SP. = 7'-01/4" 6-S1306E	3"
	2-S1307E	S2210E THRU BEAM	2-S1307E	
BEAM 8	2-S2208E S1909E	NEW BEAM 7 S2210E	S1909E 2-S2208E	BEAM 6
EASTSIDE	TRANSVERSE SECTIO	ON THRU CONCRETE END	DIAPHRAGM	CUT & REMOVE INPLACE REINFORCEMENT. CLEAN & STRAIGHTEN. LAP 2'-0" ±

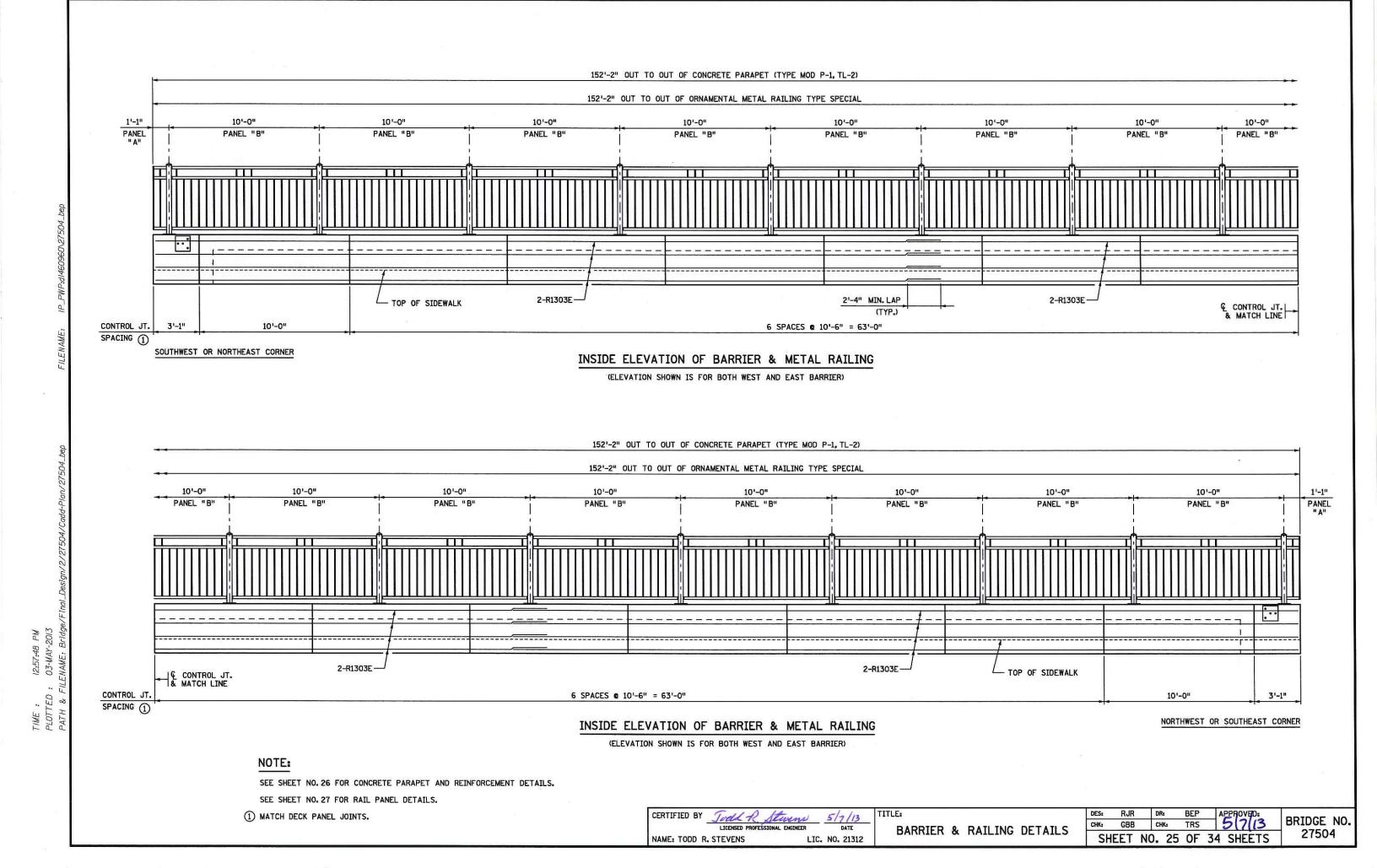
SOUTH ABUTMENT

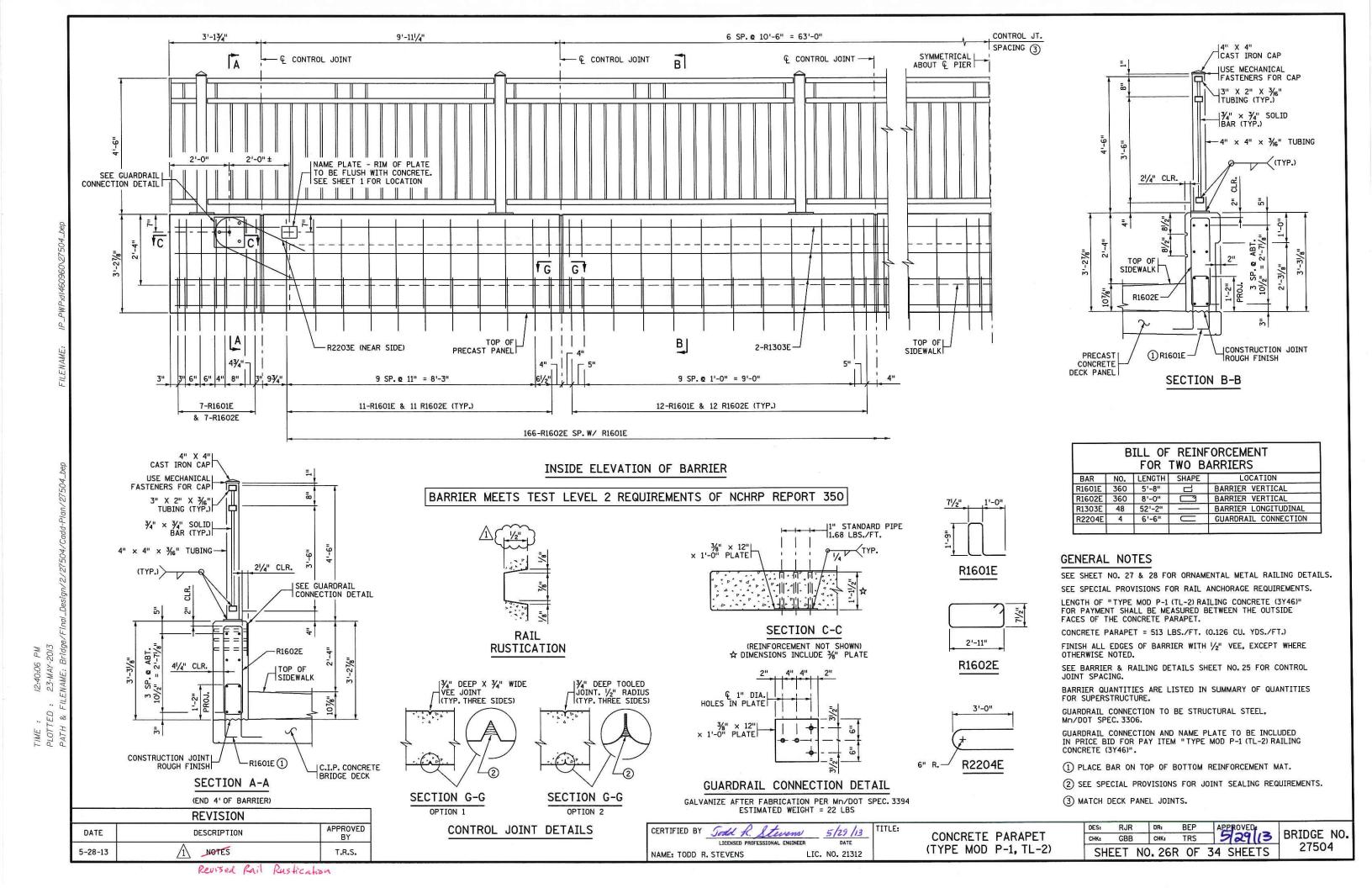
	REVISION	
DATE	DESCRIPTION	APPROVED BY
5-28-13	ADD SECTION AT PIER	TRS

	CERTIFIED BY	Total R.	Stevens		5/29/13	TITLE:
		LICENSED PROFESSIONAL ENGINEER			DATE	SUPE
NAME: TODD R		. STEVENS LIC.		NO. 21312	301 L	

3	TITLE:	
2	SUPERSTRUCTURE	DETAILS

DES:	RJR	DR:	BEP	APPROVED:	DDIDOE NO
CHK:	GBB	CHK:	TRS	5/29/13	BRIDGE NO.
SHE	EET NO	. 24	R OF	34 SHEETS	27504

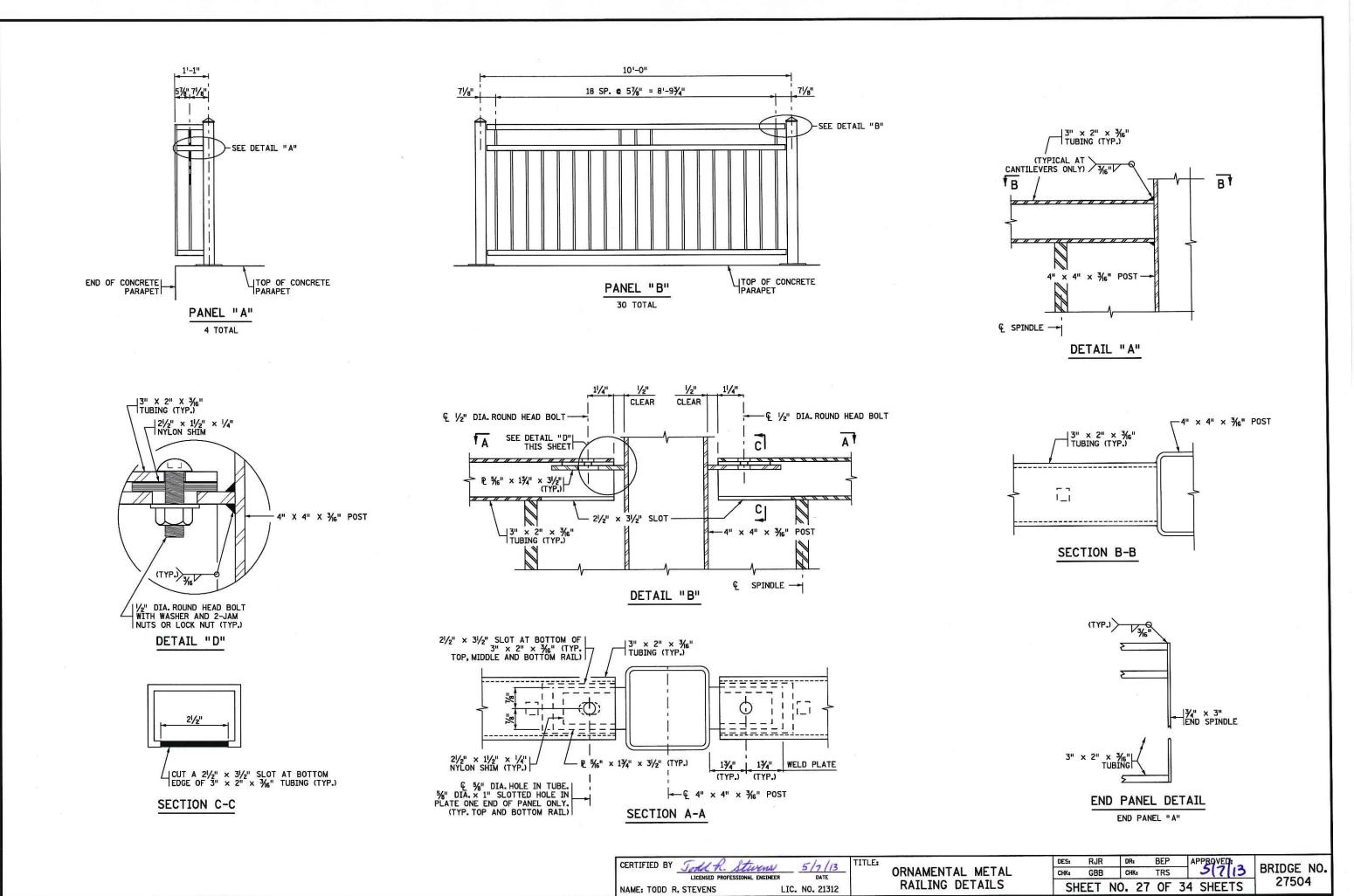












GENERAL NOTES:

ALL STRUCTURAL STEEL TUBING IN THE RAIL SHALL BE ASTM A 500, GRADE B AND SHALL CONFORM TO MINDOT SPEC. 3361. ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO MINDOT SPEC. 3306.

ANCHOR RODS SHALL CONFORM TO MODOT SPEC. 3385, TYPE A. SEE SPECIAL PROVISIONS FOR RAIL ANCHORAGE REQUIREMENTS.

VENT HOLES SHALL BE DRILLED IN THE RAIL POST BASE AND THE RAIL TUBES AS NECESSARY TO FACILITATE GALVANIZING.

GALVANIZE BOLTS, NUTS, AND WASHERS PER SPEC. 3392.

GALVANIZE ALL OTHER STRUCTURAL STEEL PER Mn/DOT SPEC. 3394 AFTER FABRICATION.

RAIL POSTS AND PICKETS SHALL BE VERTICAL AS NOTED OR SHOWN.

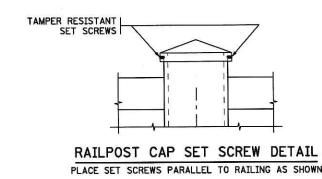
HORIZONTAL RAILS SHALL BE PARALLEL TO THE TOP OF THE PARAPET.

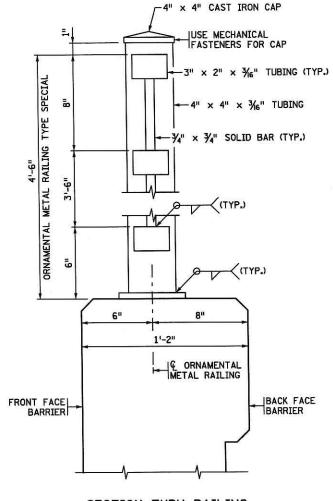
RAILING, BASE PLATES AND EXPOSED PORTIONS OF BOLTS, NUTS AND WASHERS SHALL BE PAINTED IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

PRICE BID FOR ORNAMENTAL METAL RAILING INCLUDES THE ANCHORAGES AND ALL MATERIAL ABOVE TOP OF CONCRETE PARAPET.

LENGTH OF ORNAMENTAL METAL RAILING FOR PAYMENT SHALL BE MEASURED BETWEEN THE OUTSIDE ENDS OF THE ORNAMENTAL METAL RAILING.

SEE SHEET 26 FOR ADDITIONAL DETAILS AND NOTES.



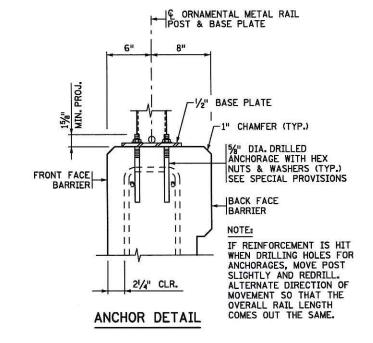


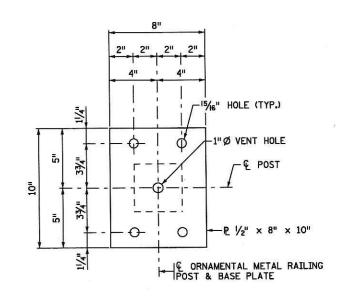
SECTION THRU RAILING

5(7(13

BRIDGE NO.

27504



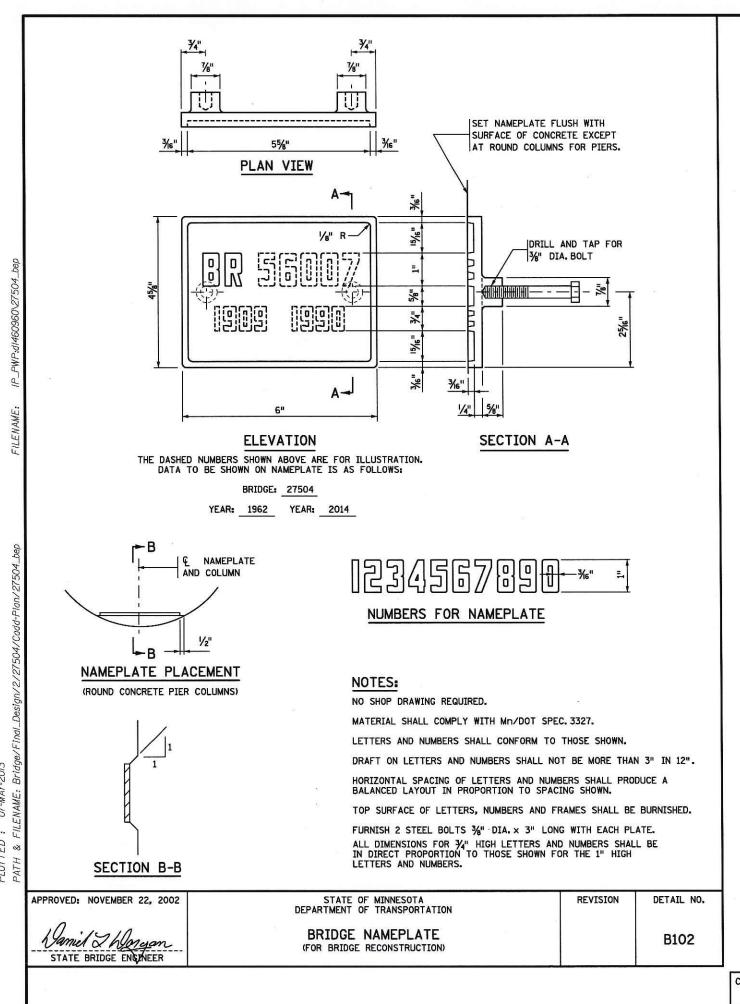


BASE PLATE

RAIL POST 13%" WEEP HOLE ON OUTSIDE

WEEPHOLE DETAIL

Todd R. Stevens 5/7/13 ORNAMENTAL METAL CHK: GBB CHK: TRS RAILING DETAILS SHEET NO. 28 OF 34 SHEETS NAME: TODD. R. STEVENS LIC. NO. 21312



CERTIFIED BY Jodl R. Stevens 5/1/13 TITLE:

NAME: TODD R. STEVENS LIC. NO. 21312

DETAILS

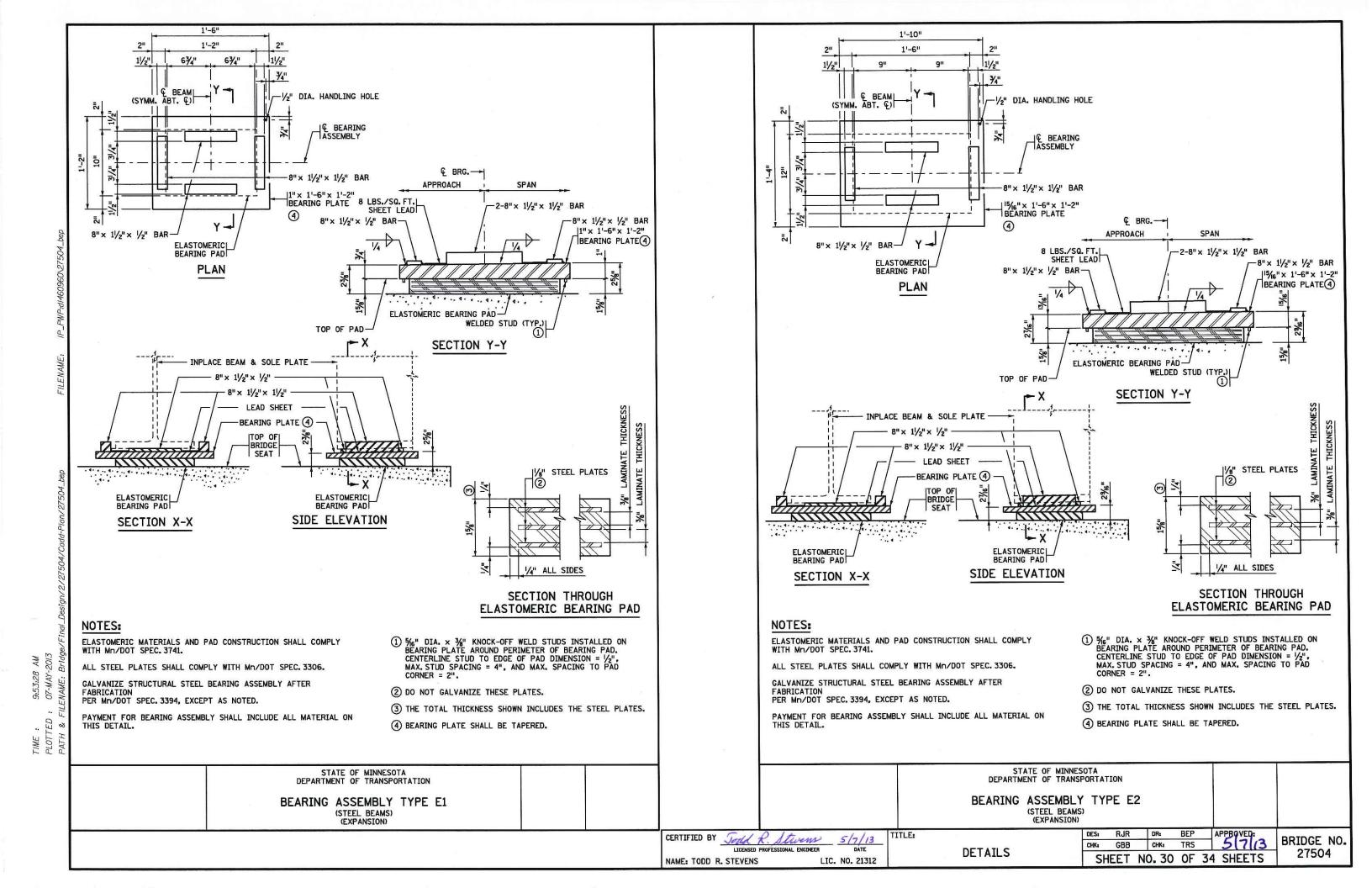
DESS RJR DR: BEP APPROVED: 5/1/13 TRS 5/1/13 SHEET NO. 29 OF 34 SHEETS

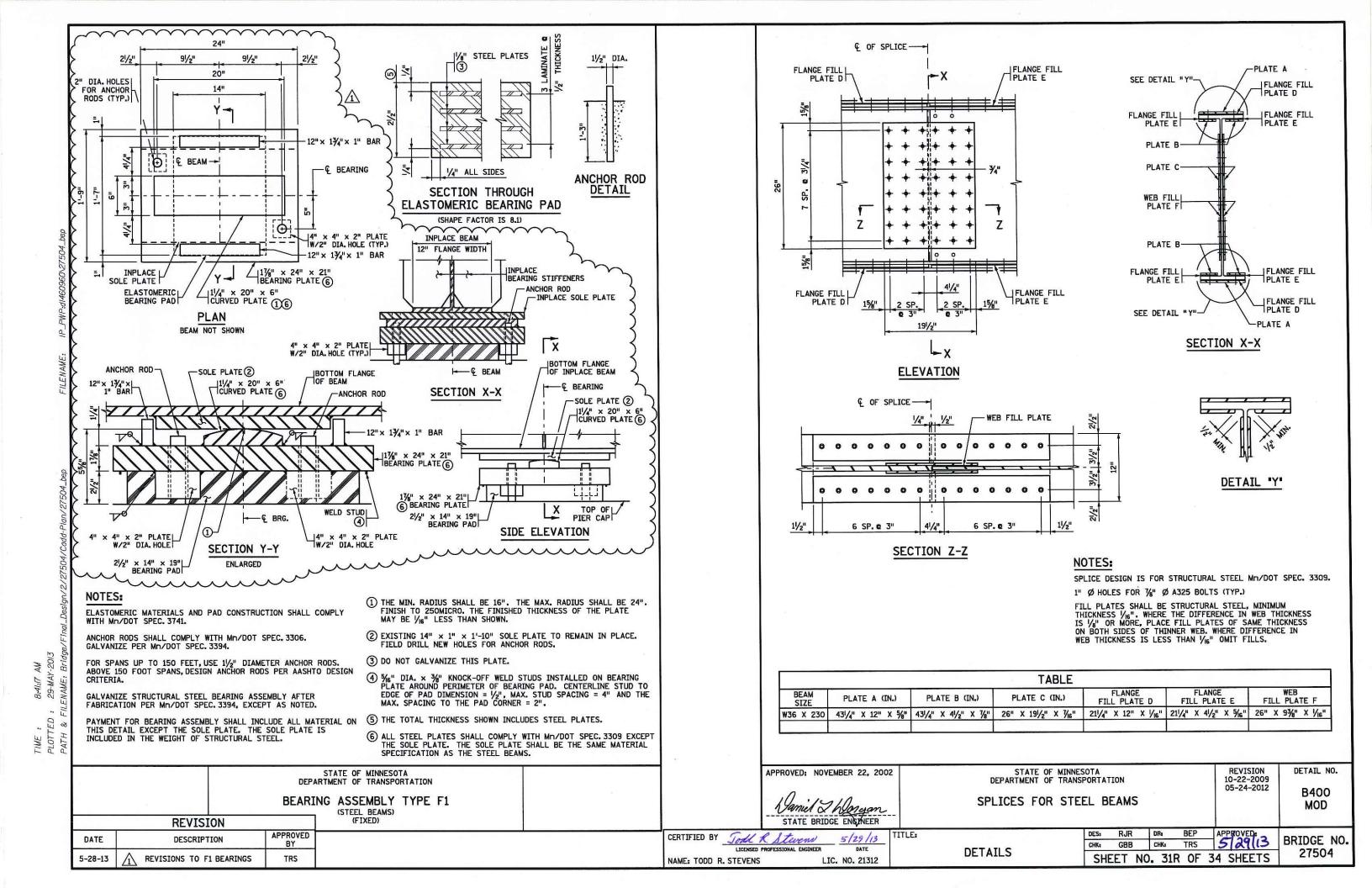
DETAILS

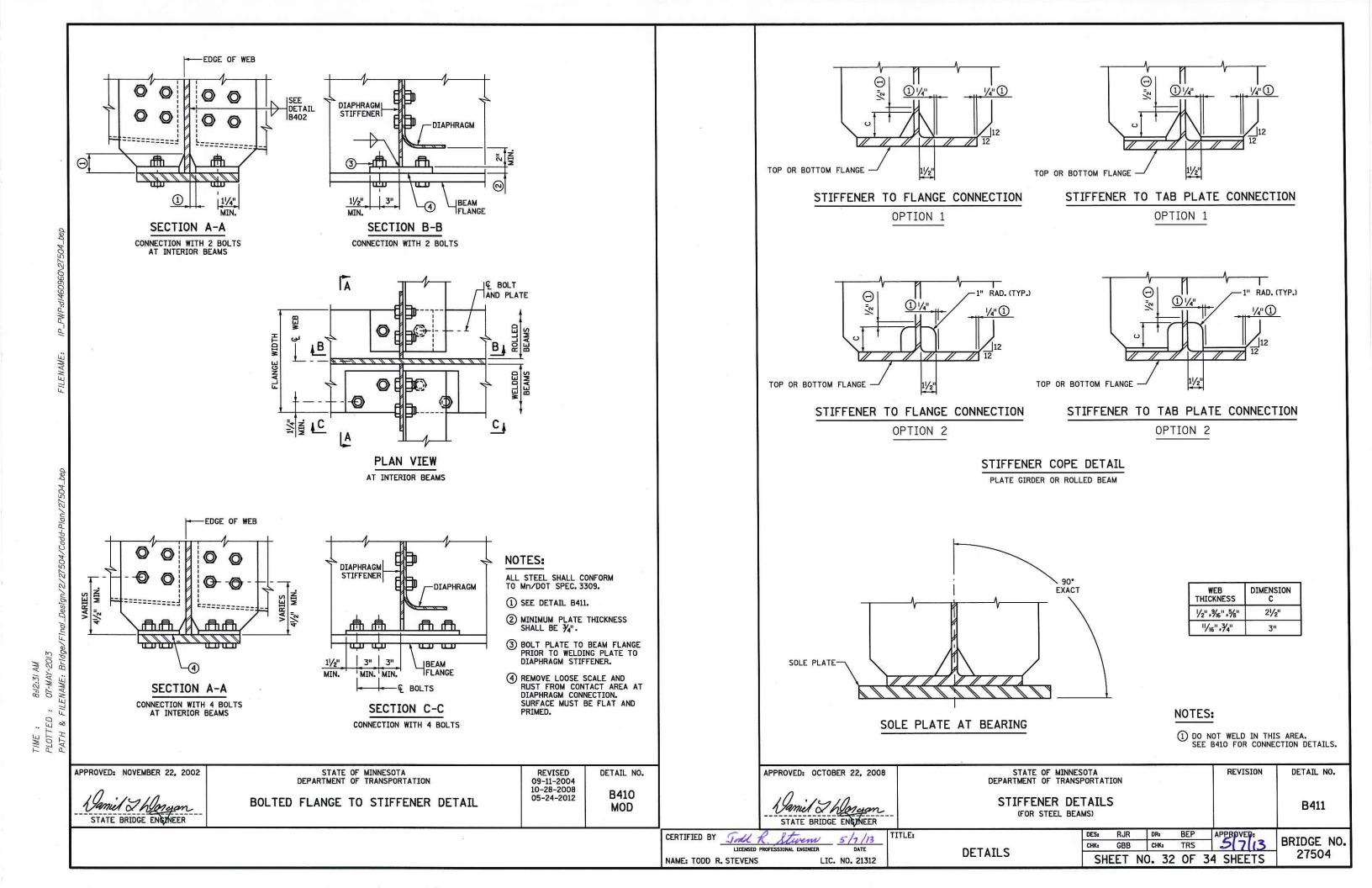
DETAILS

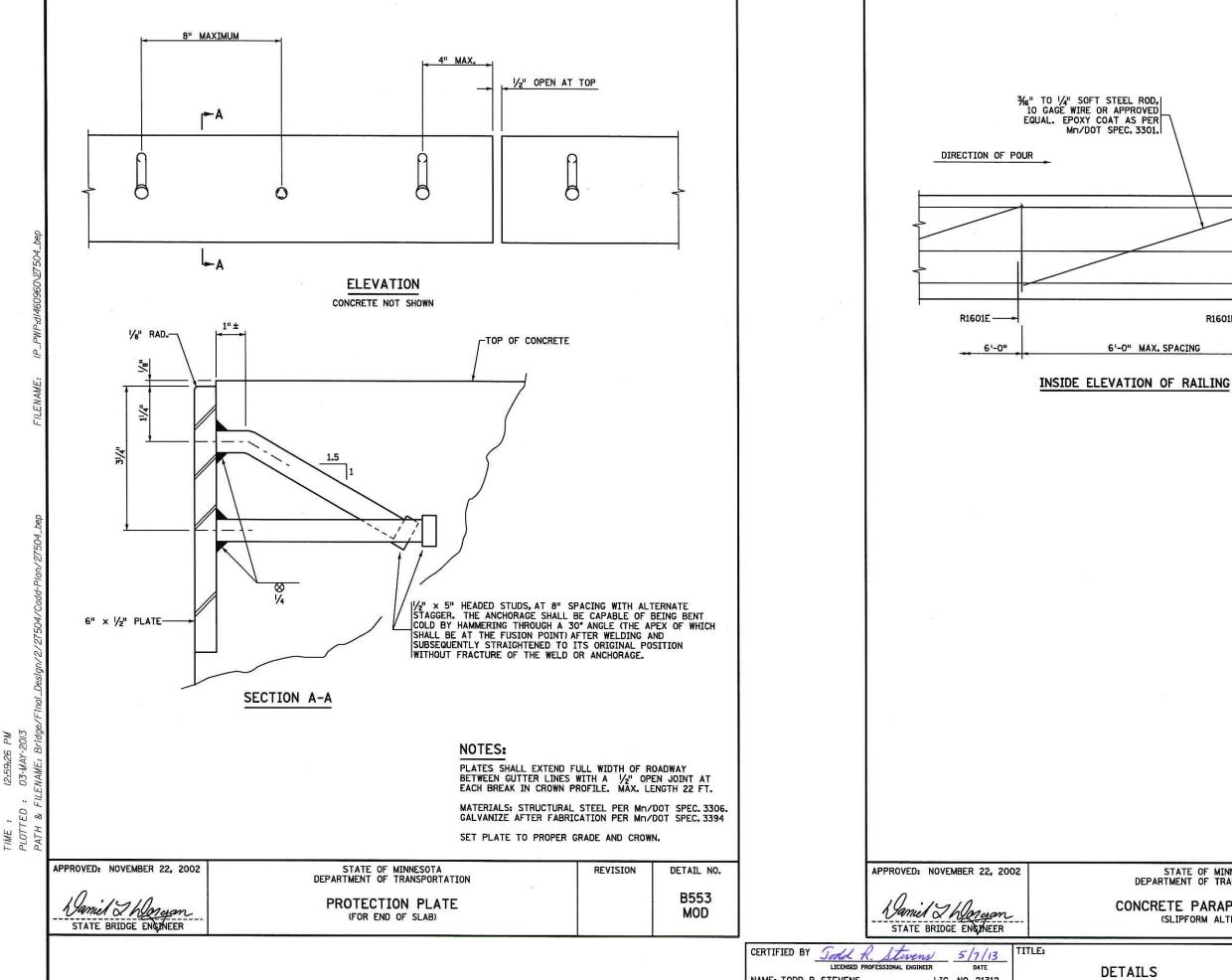
DETAILS

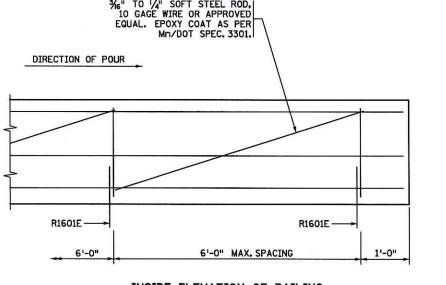
DESS RJR DR: BEP APPROVED: 5/1/13 SHEET NO. 29 OF 34 SHEETS

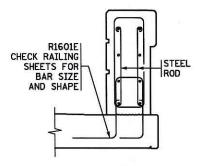












RAILING SECTION ORNAMENTAL METAL RAILING NOT SHOWN

NOTES:

CONTRACTOR WILL TOOL V-GROOVE AT CONTROL JOINTS AT TIME RAIL IS CAST AND SHALL EXTEND V-GROOVE AROUND ENTIRE PERIMETER OF RAIL.

FOR ADDITIONAL DIMENSIONS, DETAILS, REINFORCEMENT AND NOTES SEE RAILING SHEET.

FORM RAIL FOR A MINIMUM OF 2'ON EACH SIDE OF EXPANSION DEVICES, LIGHT STANDARDS AND DECK DRAIN BOX OUTS.

PAY QUANTITIES WILL NOT BE ADJUSTED AS A RESULT OF SELECTING THIS ALTERNATE.

USE A SIMILAR METHOD FOR TALLER RAILINGS OR MODIFIED VERSIONS OF THIS RAILING.

27504

APPROVED: NOVEMBER 22, 2002 STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION						REVISION	DETAIL NO.		
Damiel The STATE BRIDGE		٤	CONCRETE P. (SLIPFOR	ARAPET RAME (M. ALTERNATE)	AILING				B831 MOD
R. Stevens :	5/7/13	TITLE:		DES	RJR	DRs	BEP	APPROVED:	BRIDGE NO.
PROFESSIONAL ENGINEER	DATE		DETAILS	CHK:	GBB	CHK:	TRS	12/11/2	27504

GBB CHK: SHEET NO. 33 OF 34 SHEETS NAME: TODD R. STEVENS LIC. NO. 21312

APPROVED: SEPTEMBER 26, 2003

(AS NEEDED)

AS-BUILT BRIDGE DATA

BEP CHK: TRS CHK: SHEET NO. 34 OF 34 SHEETS

BRIDGE NO. 27504