

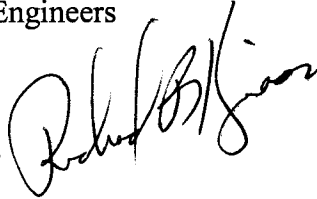
**MINNESOTA DEPARTMENT OF TRANSPORTATION  
STATE AID FOR LOCAL TRANSPORTATION DIVISION**

**Technical Memorandum No. 04-SA-04**

**September 20, 2004**

**TO:** County Engineers  
City Engineers  
District State Aid Engineers

**FROM:** Julie Skallman  
State Aid Engineer



**SUBJECT:** Hydraulic Risk Assessment Form

The Risk Assessment Form was created in 1981 as a response to FHPM 6.7.3.2 (Currently FAPG 23 CFR 650A) "Location and Hydraulic Design of Encroachments on Flood Plains". The intent of the Regulation was to encourage the abolishment of arbitrary design frequencies, such as the 50 year frequency, for all classes of roads. Instead, it was decided that the design selected for an encroachment should be supported by analyses of design alternatives with consideration given to capital costs and risks to other economic, engineering, social and environmental concerns. The process to determine the least total expected cost (LTEC) requires considerable expenditure of resources. MnDOT developed a risk assessment procedure as an attempt to screen the projects and determine the level of analysis required.

Question #4 on the Risk Assessment Form addresses capital cost of the structure. It currently states that if the cost of the structure exceeds \$500,000 an LTEC Design processes is specified or justification as to why it is not required is necessary. This dollar amount was based on \$45/sq ft for the bridge structure back in 1981. The current cost for building a bridge structure has significantly increased. Average bridge costs ranged from \$75 to \$95/ sq ft in the Annual Federal Report for fiscal year 2003. Therefore the structural cost limit is raised to \$1 million effective immediately. A copy of the revised form is attached for your use. The form may be downloaded from the SALT website at [http://www.dot.state.mn.us/stateaid/res\\_sa\\_tech\\_memos.html](http://www.dot.state.mn.us/stateaid/res_sa_tech_memos.html). Look for Tech Memo 04-SA-04 on the list on that page.

If you have any questions concerning this memorandum, please contact Paul Stine, State Aid Operations Engineer, at (651) 296-9973. Any questions regarding the attached form should be directed to Petra DeWall, Hydraulic Design Engineer, at (651) 747-2164.

Enclosure: Risk Assessment for Encroachment Design

cc: DSAA

# RISK ASSESSMENT FOR ENCROACHMENT DESIGN

Date \_\_\_\_\_

District \_\_\_\_\_

County \_\_\_\_\_

Vicinity of \_\_\_\_\_

Sec. \_\_\_\_ T \_\_\_\_ R \_\_\_\_

## DATA REQUIREMENTS

1. Location of Crossing: Roadway \_\_\_\_\_ C.S. \_\_\_\_ M.P. \_\_\_\_

2. Name of Stream: \_\_\_\_\_ Bridge No. Old: \_\_\_\_\_ New: \_\_\_\_\_

3. Current ADT \_\_\_\_\_ Projected ADT \_\_\_\_\_

4. Practicable detour available Yes \_\_\_\_\_ No \_\_\_\_\_

If no is checked, please explain: \_\_\_\_\_

If there is no practicable detour available, then the use of the road must be analyzed. Considerations such as emergency vehicle access, emergency supply and evacuation route, and the need for school bus, milk and mail routes should be studied. Factors to consider for this analysis include design frequency, depth, duration, and frequency of inundation if appropriate, and available funding.

5. Hydraulic Data: (Fill in as appropriate)

Approximate Flowline Elevation

$Q_2 =$  \_\_\_\_\_

TW<sub>2</sub> Elevation \_\_\_\_\_

$Q_5 =$  \_\_\_\_\_

TW<sub>5</sub> Elevation \_\_\_\_\_

$Q_{10} =$  \_\_\_\_\_

TW<sub>10</sub> Elevation \_\_\_\_\_

$Q_{25} =$  \_\_\_\_\_

TW<sub>25</sub> Elevation \_\_\_\_\_

$Q_{50} =$  \_\_\_\_\_

TW<sub>50</sub> Elevation \_\_\_\_\_

$Q_{100} =$  \_\_\_\_\_

TW<sub>100</sub> Elevation \_\_\_\_\_

Circle Design Frequency

Reasons for selecting Design Frequency: \_\_\_\_\_

6. Magnitude and Frequency of the smaller of "Overtopping" or "500 yr." flood:

\_\_\_\_\_ cfs \_\_\_\_\_ year frequency

7. Low member elevation \_\_\_\_\_

8. Minimum roadway overflow elevation if appropriate \_\_\_\_\_

9. Elevation of high risk property, i.e. residences \_\_\_\_\_

Other buildings \_\_\_\_\_

10. Horizontal location of overflow:

At structure \_\_\_\_\_ (See 12) Not at structure \_\_\_\_\_

11. Type of proposed structure:

Bridge \_\_\_\_\_ (See 12) Culvert(s) \_\_\_\_\_

12. If the proposed structure is a bridge with the sag point located on the bridge and there is ice and debris potential, strong consideration should be given to using  $Q_{50}$  as design discharge with 3' of clearance between the 50 year tailwater stage and low member.

DATA  
REQUIREMENTS

LTEC  
DESIGN

1. **BACKWATER DAMAGE** - Major flood damage in this context refers to shopping centers, hospitals, chemical plants, power plants, housing developments, etc.

1a. Is the overtopping flood greater than the 100 yr. flood?

Yes \_\_\_(Go to 1 b.) No \_\_\_(Go to 1 e.)

1b. Is the overtopping flood greater than the "greatest" flood (500 yr. frequency)?

Yes \_\_\_(Go to 1 d.) No \_\_\_(Go to 1 c.)

1c. Is there major flood damage potential for the overtopping flood?

No \_\_\_(Go to 1 e.)

1d. Is there major flood damage potential for the "greatest" flood (500 year frequency)?

No \_\_\_(Go to 1 e.)

1e. Will there be flood damage potential to residence(s) or other buildings during a 100 yr. flood?

Yes \_\_\_(Go to 1 f.) No \_\_\_(Go to 2)

1f. Could this flood damage occur even if the roadway crossing wasn't there?

Yes \_\_\_(Go to 1 g.) No \_\_\_(Go to 1 h.)

1g. Could this flood damage be significantly increased by the backwater caused by the proposed crossing?

Yes \_\_\_(Go to 1 h.) No \_\_\_(Go to 2)

1h. Could the stream crossing be designed in such a manner so as to minimize this potential flood damage?

Yes \_\_\_(Go to 1 i.) No \_\_\_(Go to 2)

YES \_\_\_  
(Go to 1 e.)

YES \_\_\_  
(Go to 1 e.)

**DATA  
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li. Does the value of the building(s) and/or its contents have sufficient value to justify further evaluation of risk and potential flood damage?  
No \_\_\_(Go to 2)

**LTEC  
DESIGN**

YES \_\_\_  
(Go to 2)

**2. TRAFFIC RELATED LOSSES**

2a. Is the overtopping flood greater than the "greatest" flood (500 yr. frequency)?  
Yes \_\_\_(Go to 3) No \_\_\_(Go to 2 b.)

2b. Does the ADT exceed 50 vehicles per day?  
Yes \_\_\_(Go to 2 c.) No \_\_\_(Go to 3)

2c. Would the (duration of road closure in days) multiplied by the (length of detour minus the length of normal route in miles) exceed 20?  
Yes \_\_\_(Go to 2 d.) No \_\_\_(Go to 3)

2d. Does the annual risk cost for traffic related costs exceed 10% of the annual capital costs?  
No \_\_\_(Go to 3) (See figures A and B for assistance)

YES \_\_\_  
(Go to 3)

**3. ROADWAY AND/OR STRUCTURE REPAIR COSTS**

3a. Is the overtopping flood less than a 100 year frequency flood?  
Yes \_\_\_(Go to 3 b.) No \_\_\_(Go to 3 i.)

3b. Compare the tailwater (TW) elevation with the roadway sag point elevation for the overtopping flood. Check the appropriate category.

When TW is above the sag point (Go to 4)

When TW is between 0 and .5N below sag point (Go to 3 c.)

When TW is between .5N and 1.0N below sag point (Go to 3 d.)

When TW is 1.0N and 2.0N below sag point (Go to 3 e.)

When TW is more than 2.0N below sag point (Go to 3 g.)

3c. Does the embankment have a good erosion resistant vegetative cover?  
Yes \_\_\_(Go to 3 i.) No \_\_\_(Go to 3 d.)

3d. Is the shoulder constructed from erosion resistant material such as paved, coarse gravel, or clay type soil?  
Yes \_\_\_(Go to 3 i.) No \_\_\_(Go to 3 e.)

3e. Will the duration of overtopping for the 25 year flood exceed 1 hour?  
Yes \_\_\_(Go to 3 f.) No \_\_\_(Go to 3 i.)

3f. Is the embankment constructed from erosion resistant material such as a clay type soil?  
Yes \_\_\_(Go to 3 i.) No \_\_\_(Go to 3 g.)

3g. Is the overtopping flood less than a 25 year frequency flood?  
Yes \_\_\_(Go to 3 h.) No \_\_\_(Go to 3 i.)

3h. Will the cost of protecting the roadway and/or embankment from severe damage

YES \_\_\_

DATA  
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caused by overtopping exceed the cost of providing additional culvert or bridge capacity?  
No \_\_\_(Go to 3 i.)

(Go to 3 i.)

3i. Is there damage potential to the structure caused by scour, ice, debris or other means during the lesser of the overtopping flood or the 100 year flood?  
Yes \_\_\_(Go to 3 j.) No \_\_\_(Go to 4)

3j. Will the cost of protecting the structure from damage exceed the cost of providing additional culvert or bridge water capacity?  
No \_\_\_(Go to 4)

YES \_\_\_  
(Go to 4)

4. Will the capital cost of the structure exceed \$1,000,000?  
No \_\_\_(Go to 5)

YES \_\_\_  
(Go to 5)

5. In your opinion, are there any other factors you feel should require further study through a risk analysis?  
No \_\_\_(Go to 6)

YES \_\_\_  
(Indicate)

6. If there are no TNs in the LTEC Design column on the right, proceed with the design, selecting the lowest acceptable grade line and the smallest waterway opening consistent with the constraints imposed on the project. The risk assessment has demonstrated that potential flood damage costs, traffic related costs, roadway and/or structure repair costs are minor and therefore disregarded for this project.

One or more TNs in the LTEC Design column indicates further analysis in the category checked may be required utilizing the LTEC design process or justification why it is not required.

JUSTIFICATION

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I hereby certify that this plan, specification, or report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota

Signature: \_\_\_\_\_

Printed Name: \_\_\_\_\_

License Number: \_\_\_\_\_

Date: \_\_\_\_\_