

Permeability of Base Aggregate and Sand Project TPF 5(341)

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National Road Research Alliance

STRATEGIC IMPLEMENTATION THROUGH COOPERATIVE PAVEMENT RESEARCH

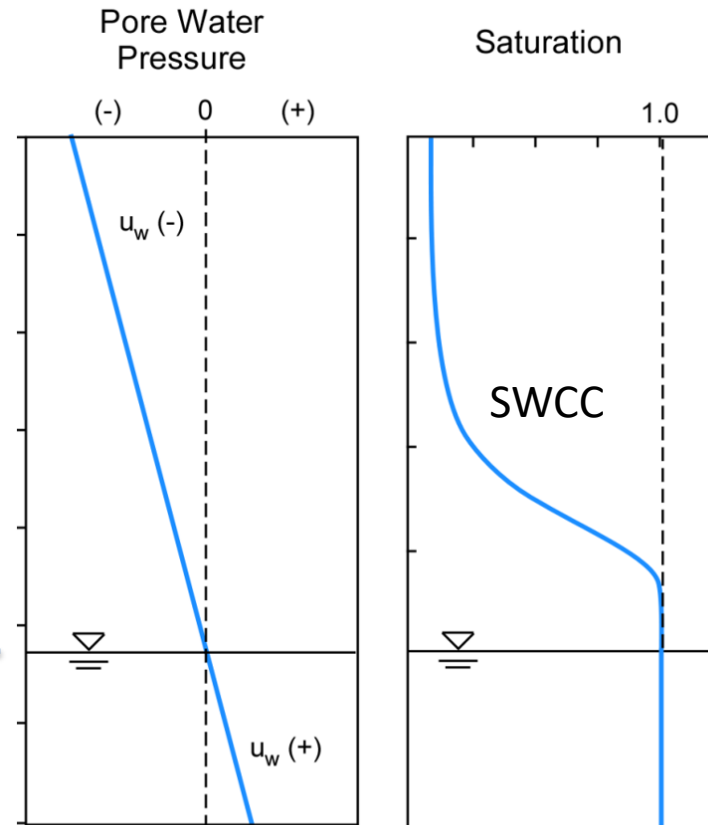
Statement of Problem

- Lack of proper pore water drainage is one of the main causes of geosystem failure (e.g., roadway base course, retaining wall backfills).
- Proper drainage required to minimize elevated pore pressure, minimize freeze-thaw damage.
- Simple and reliable tools capable of estimating drainability values for common aggregate types will aid in material selection and design
 - Saturated hydraulic conductivity, K_{sat}
 - SWCC parameters

Project Objectives

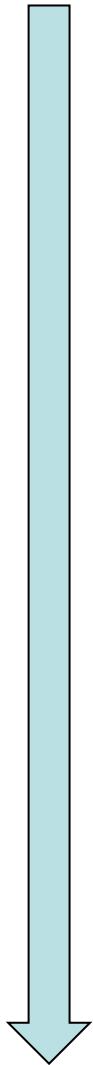
- Assess permeability of a wide range of coarse materials applicable to roadway construction.
- Conduct laboratory permeability and water retention tests on materials of different types, gradations, angularity, fine contents, and crushing percentages.
- Develop simple predictive tools that may be used to assess permeability/drainability from other properties (e.g., gradation, fines content, etc.)

- Drainability - An Unsaturated Soils Problem!



Permeability and Water Retention

Increasing Suction, Decreasing Saturation



(a)



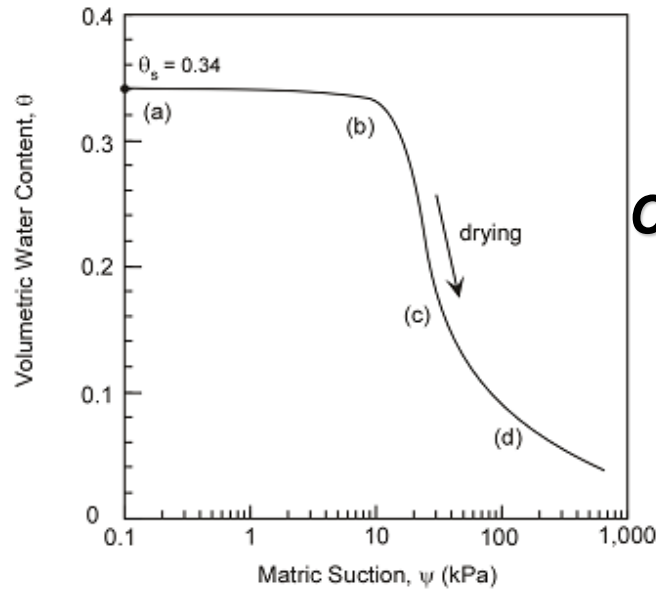
(b)



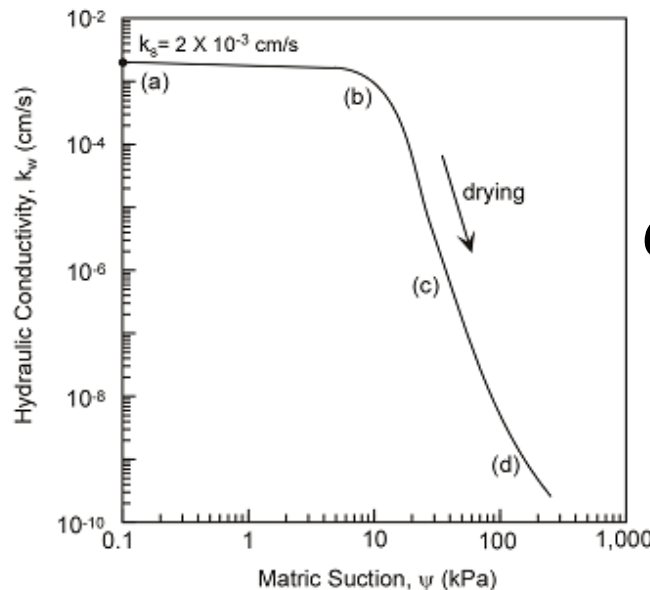
(c)



(d)



**Soil-Water
Characteristic Curve
(SWCC)**



**Hydraulic
Conductivity Function
(HCF)**

Samples Obtained

Sample Number	Sample
1	3149 Super Sand (MnDOT)
2	MN Class 5 (MnDOT)
3	1007 Type 5 DGB (MoDOT)
4	1007 Type 7 DGB (MoDOT)
5	1010 Man. Sand (MoDOT)
6	MCC Freeborn West Quarry Crushed Stone (WisDOT)
7	Lannon Lisbon Pit (North Ave.) Structural Backfill (WisDOT)
8	Lannon Lisbon Pit (Mukwonago) Structural Backfill (WisDOT)
9	Lannon Stone Product Chips (WisDOT)
10	Super Aggregate Pit Granular Backfill (WisDOT)
15	Bryan Redrock Class 5, MnDOT Pit 70006
16	Bryan Redrock Ball Diamond material, MnDOT Pit 70006
A1	1¼" Base (WisDOT)
A2	¾" Washed (WisDOT)
A3	Manufactured Sand (WisDOT)
A4	¾" Base Cs. (WisDOT)
A5	Breaker Run (limestone/dolomite) (WisDOT)



(1) 3149 Super Sand



(2) Mn Class 5

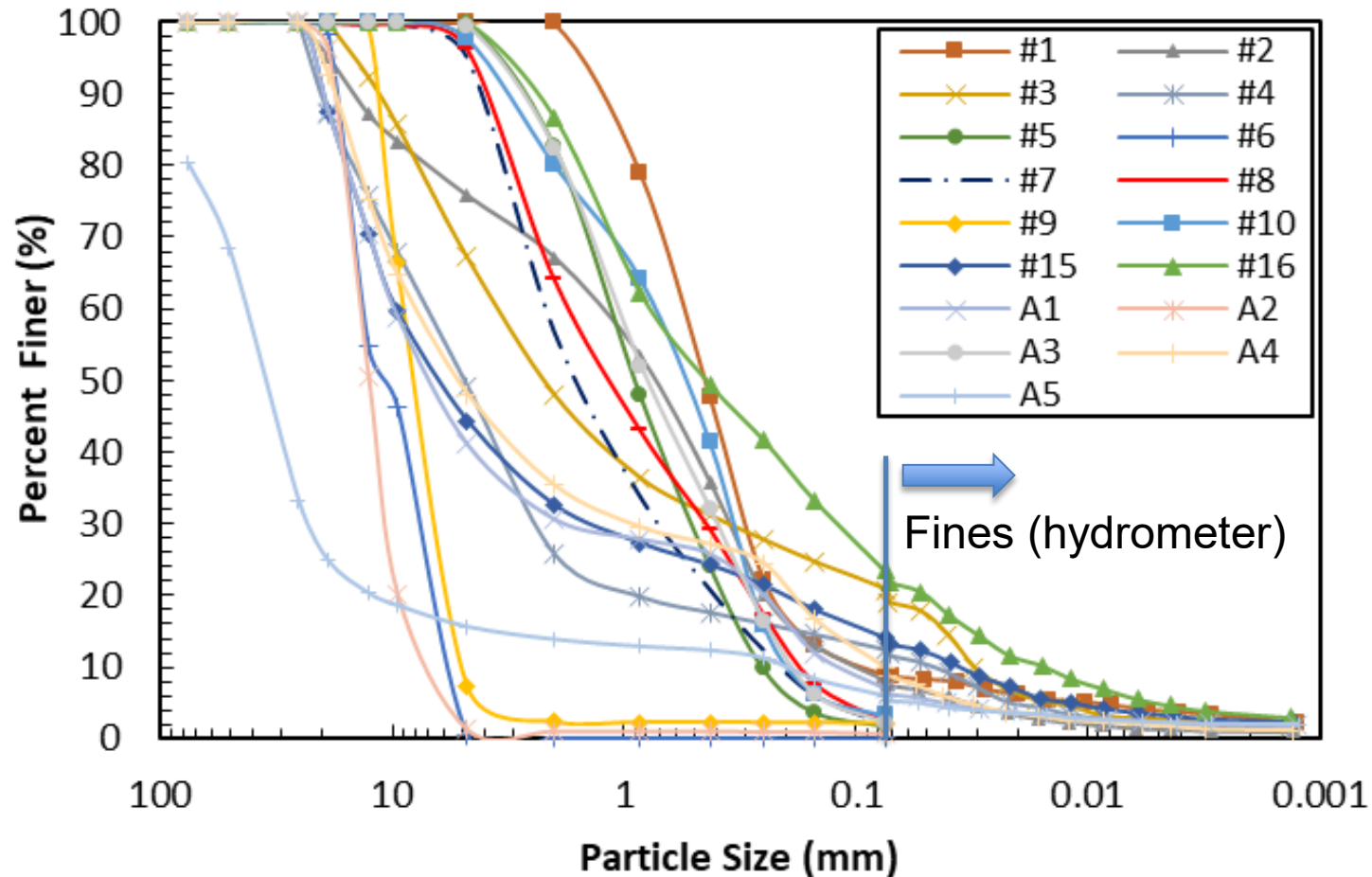


(3) 1007 Type 5 DGB



(4) 1007 Type 7 DGB

Particle Size Distribution



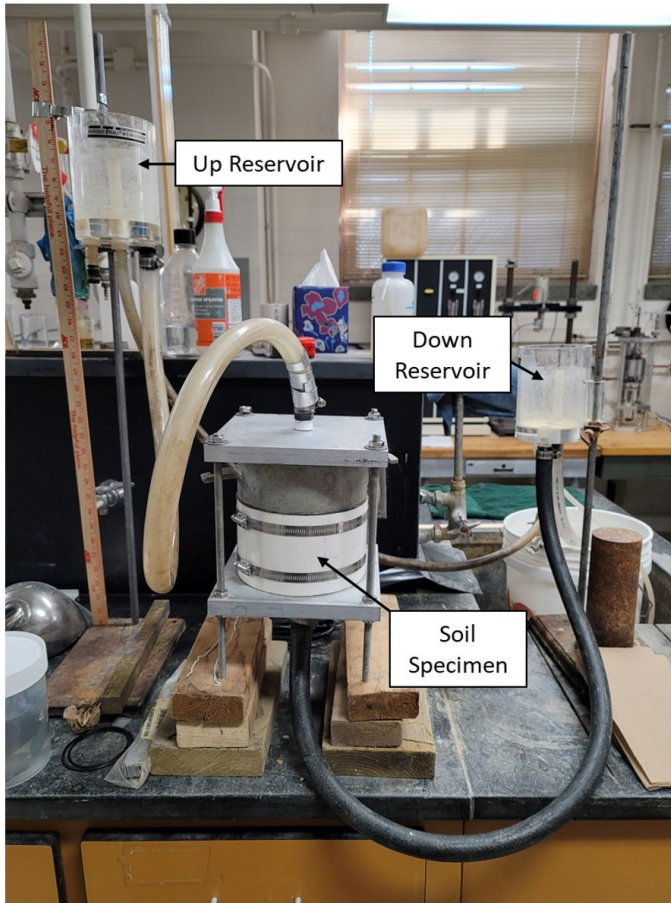
Indices and Classification

9 sands & 7 gravels; fines from ~0% to ~23%

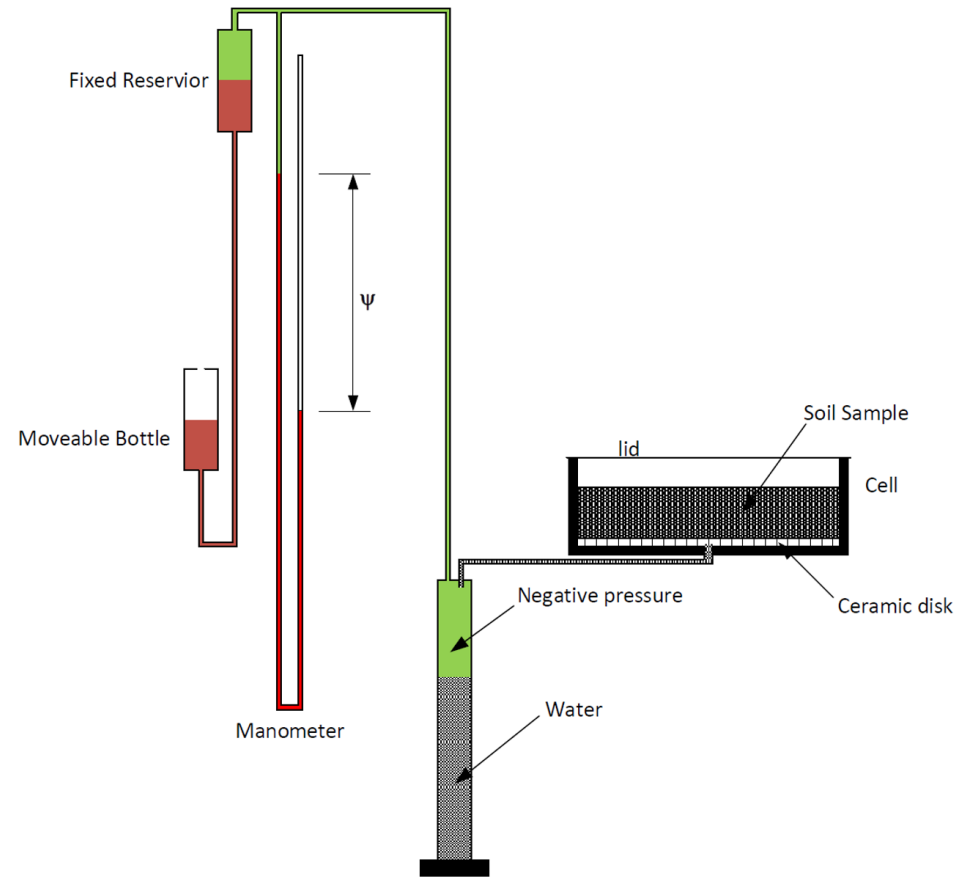
Sample	D ₁₀ (mm)	D ₃₀ (mm)	D ₅₀ (mm)	D ₆₀ (mm)	C _u	% Fines	% Gravels	γ _d (kN/m ³)	USCS
#1	0.09	0.30	0.46	0.55	5.91	8.82	100.00	18.57	SP-SM
#2	0.10	0.36	0.72	1.38	13.53	8.18	75.73	19.63	SW-SM
#3	0.03	0.36	2.28	3.65	114.78	20.8	67.18	17.76	SM
#4	0.05	2.50	4.90	7.09	154.13	12.5	49.24	17.80	GM
#5	0.27	0.52	0.90	1.26	4.75	1.89	99.75	15.91	SP
#6	5.85	7.70	10.9	14.0	2.39	0.19	0.59	16.22	GP
#7	0.22	0.72	1.70	2.20	10.09	2.03	95.17	18.65	SW
#8	0.18	0.45	1.30	1.82	10.11	2.39	96.40	20.10	SP
#9	5.00	6.60	8.05	8.95	1.79	2.17	7.37	16.57	GP
#10	0.20	0.33	0.53	0.72	3.60	3.24	97.72	18.26	SP
#15	0.04	1.58	6.35	9.50	256.76	14.1	44.37	18.66	GM
#16	0.02	0.13	0.44	0.79	46.90	23.4	99.88	17.69	SM
A1	0.13	1.82	7.00	9.92	76.31	7.05	41.03	18.63	GW-GM
A2	7.19	11.2	12.8	14.4	2.00	0.84	1.39	15.94	GP
A3	0.19	0.40	0.80	1.10	5.73	2.27	99.33	17.27	SP
A4	0.08	0.93	5.20	7.99	102.44	9.89	48.06	19.18	GW-GM

Note: D₁₀, D₃₀, D₅₀, D₆₀ = particle sizes corresponding to 10%, 30%, 50%, 60% finer, respectively, in particle-size distribution curve; C_u = coefficient of uniformity; C_c = coefficient of curvature; USCS = unified soil classification system

K_{sat} and SWCC Testing



Constant Head Apparatus
(K_{sat})

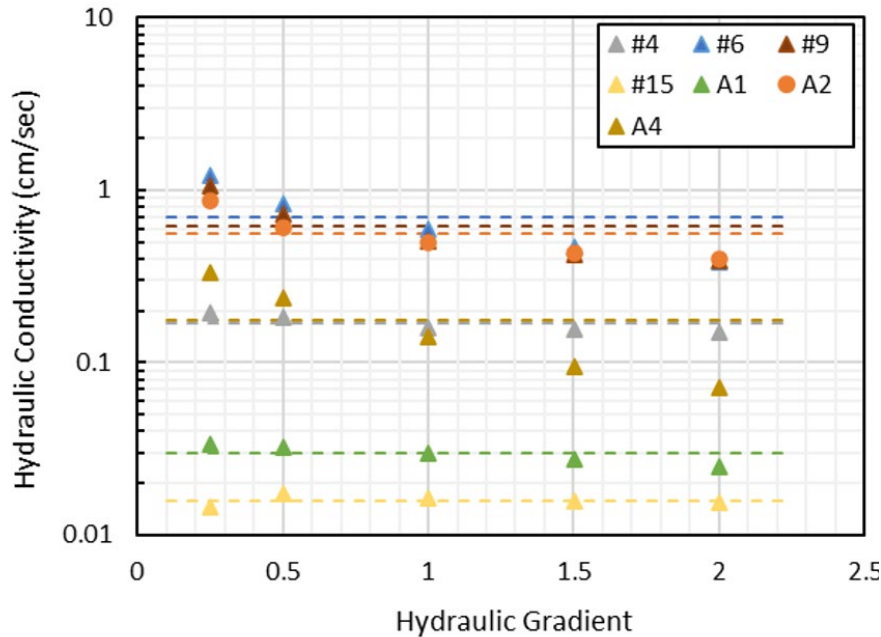


Hanging Column Apparatus
(SWCC)

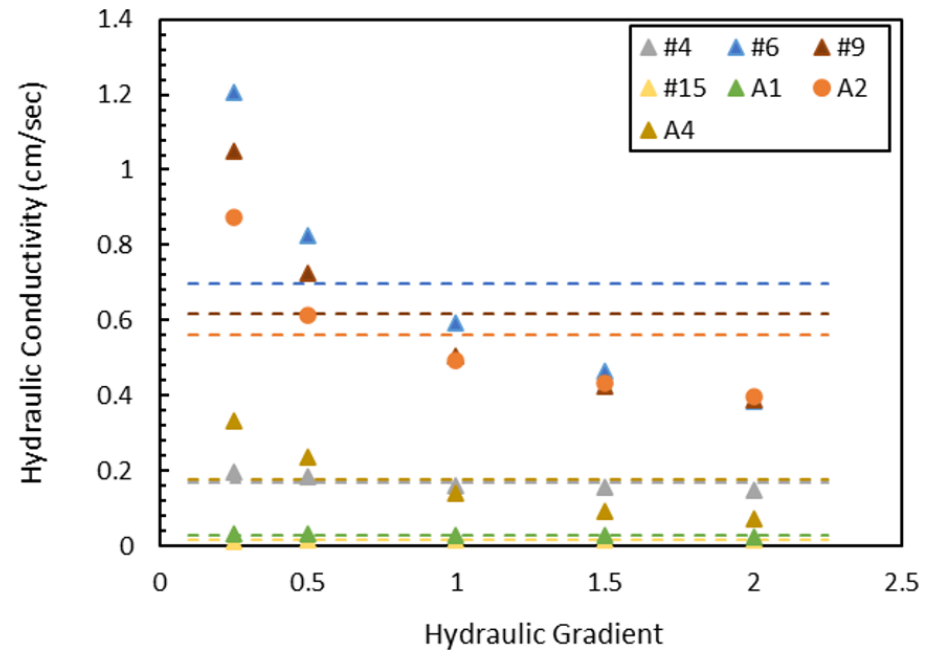
Compaction Density

Sample	Dry Unit Weight (KN/m ³)		% Difference
	K _{sat}	SWCC	
#1	18.6	18.5	0.54%
#2	19.6	19.6	0.00%
#3	17.8	17.8	0.00%
#4	17.8	17.8	0.00%
#5	15.9	16.0	0.63%
#6	16.2	16.3	0.62%
#7	18.7	18.6	0.54%
#8	20.1	20.1	0.00%
#9	16.6	16.6	0.00%
#10	18.3	18.3	0.00%
#15	18.7	18.7	0.00%
#16	17.7	17.7	0.00%
A1	18.6	18.7	0.54%
A2	15.9	16.0	0.63%
A3	17.3	17.3	0.00%
A4	19.2	19.2	0.00%

K_{sat} : Effect of Hydraulic Gradient



(Sands)



(Gravels)

- Potential fines migration
- Turbulent flow regime (gravels?)

$$Q = kiA$$

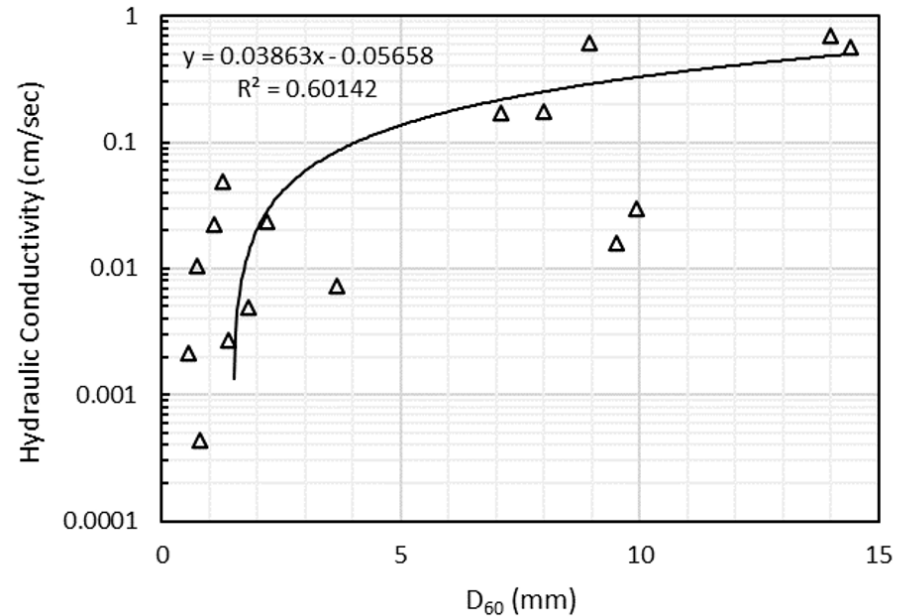
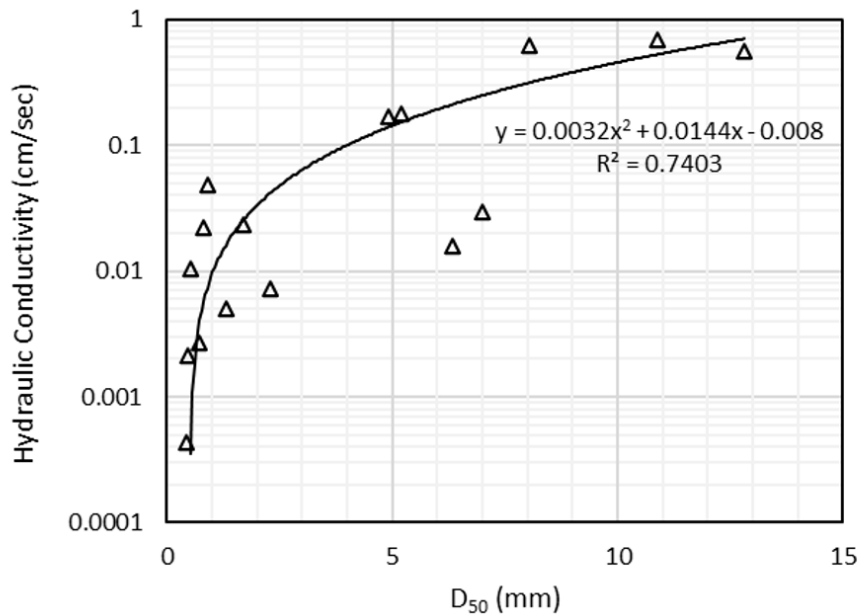
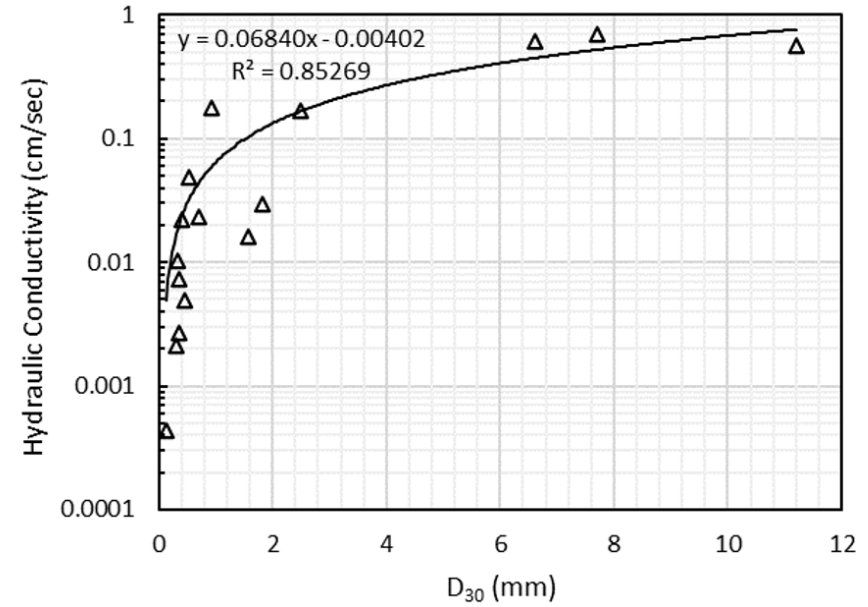
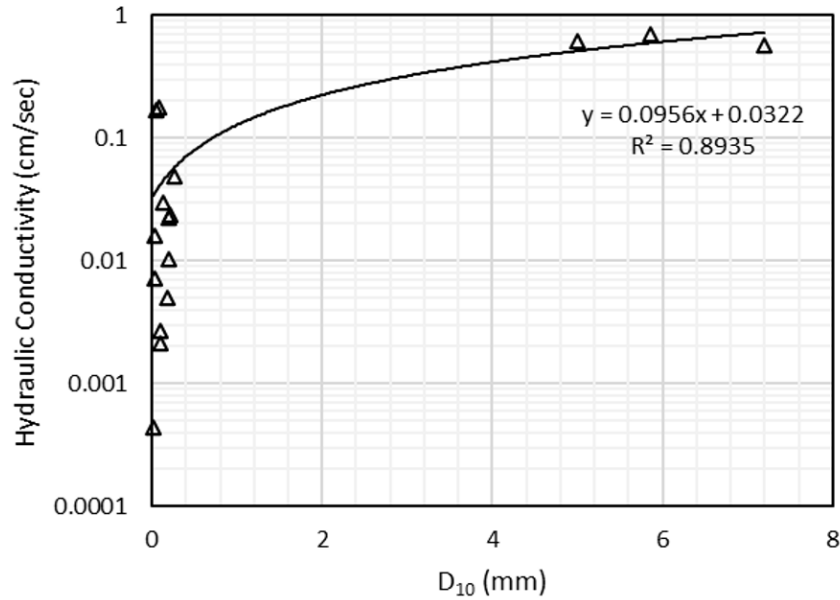
Summary of K_{sat}

Gravel average = 0.324 cm/s

Sand average = 0.014 cm/s

Sample	Hydraulic Conductivity (cm/sec)			Standard Deviation	Coefficient of Variation	
	Minimum	Maximum	Average			
#1	0.0019	0.0023	0.0021	0.0001	0.0553	
#2	0.0026	0.0030	0.0027	0.0002	0.0579	
#3	0.0068	0.0075	0.0073	0.0003	0.0367	
#4	0.1498	0.1960	0.1693	0.0180	0.1063	(% Fines)
#5	0.0479	0.0499	0.0489	0.0007	0.0148	
#6	0.3869	1.2072	0.6957	0.2960	0.4255	← GP (0.19)
#7	0.0206	0.0253	0.0236	0.0018	0.0758	
#8	0.0047	0.0052	0.0050	0.0002	0.0411	
#9	0.3889	1.0499	0.6196	0.2452	0.3957	← GP (2.17)
#10	0.0101	0.0108	0.0104	0.0002	0.0225	
#15	0.0147	0.0173	0.0159	0.0009	0.0564	
#16	0.0004	0.0005	0.0004	0.0000	0.0527	
A1	0.0251	0.0339	0.0298	0.0031	0.1043	
A2	0.3957	0.8741	0.5618	0.1725	0.3070	← GP (0.84)
A3	0.0187	0.0303	0.0224	0.0045	0.2030	
A4	0.0716	0.3327	0.1759	0.0972	0.5528	← GW-GM (9.89)

$(K_{sat})_{avg}$ correlation with grain size



Existing Empirical Equations for K_{sat}

Literature search was conducted to summarize equations for predicting saturated hydraulic conductivity from grain size, compaction indices, and fluid properties (e.g., d_{10} , porosity, viscosity)

The equations used in this study were as follows:

Alyamani and Sen (Equation B.21)

Beyer (Equation B.12)

Harleman et al. (Equation B.11)

Original Hazen (Equation B.1a)

Modified Hazen (Equation B.1b)

Kozeny (Equation B.9)

Kozeny-Carman (Equation B.10)

Sauerbrei (Equation B.19)

Slichter (Equation B.3)

Terzaghi (Equation B.5)

U.S. Bureau of Reclamation (Equation B.20)

Salarashayeri and Siosemarde (Equation B.25)

Chapuis (Equation B.28)

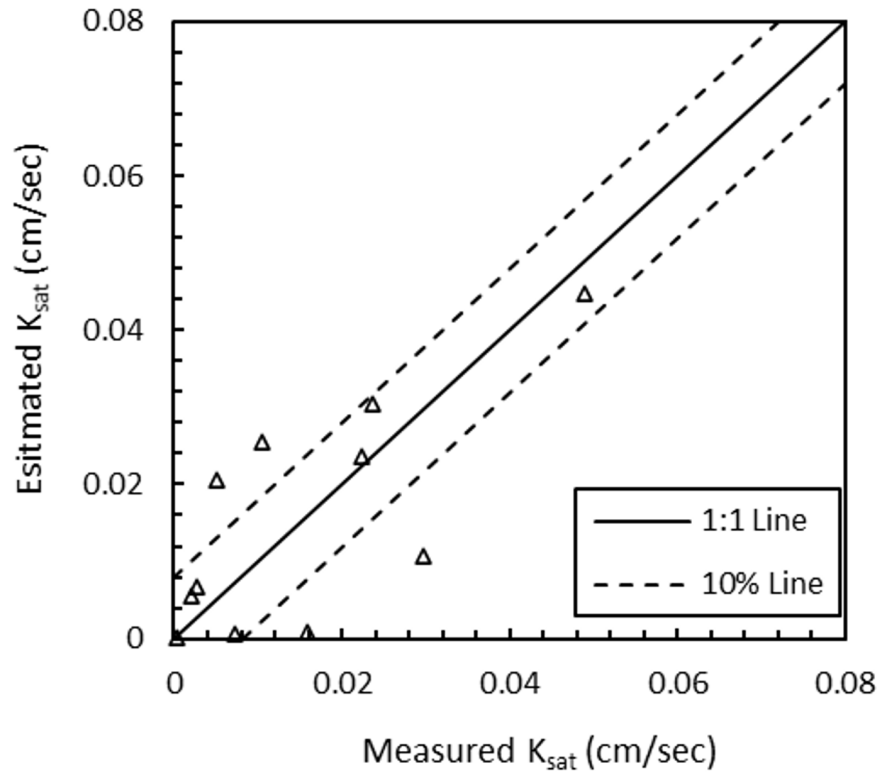
$$K[m/s] = \beta \frac{g}{\nu} \log \frac{500}{c_u} d_{10}^2$$

(Beyer, 1964)

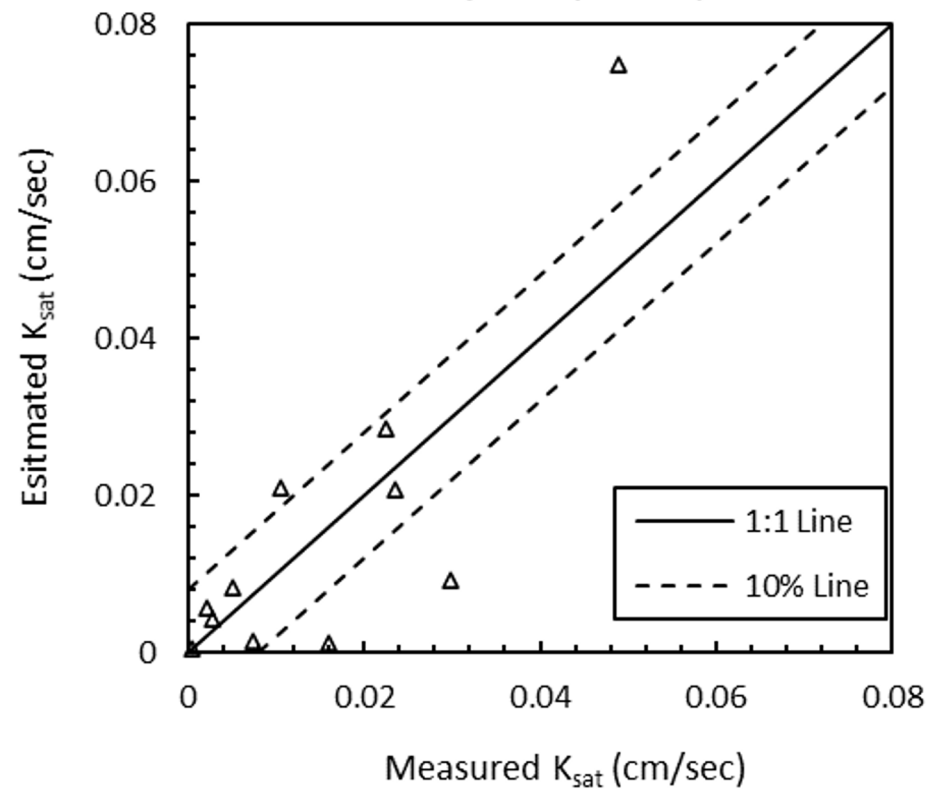
Empirical Equation Performance

(Gravels excluded)

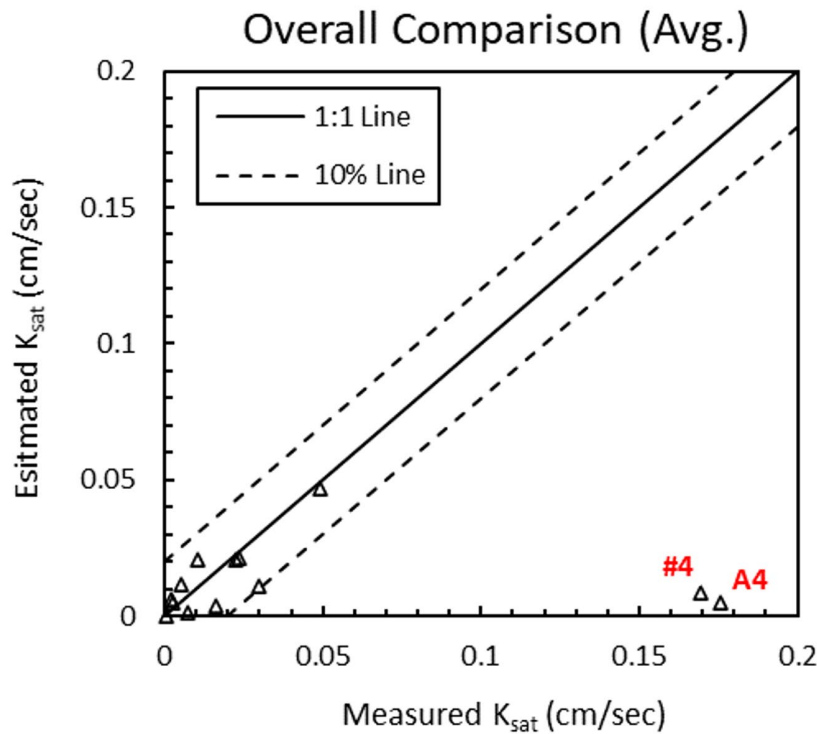
Harleman et al. (1963)



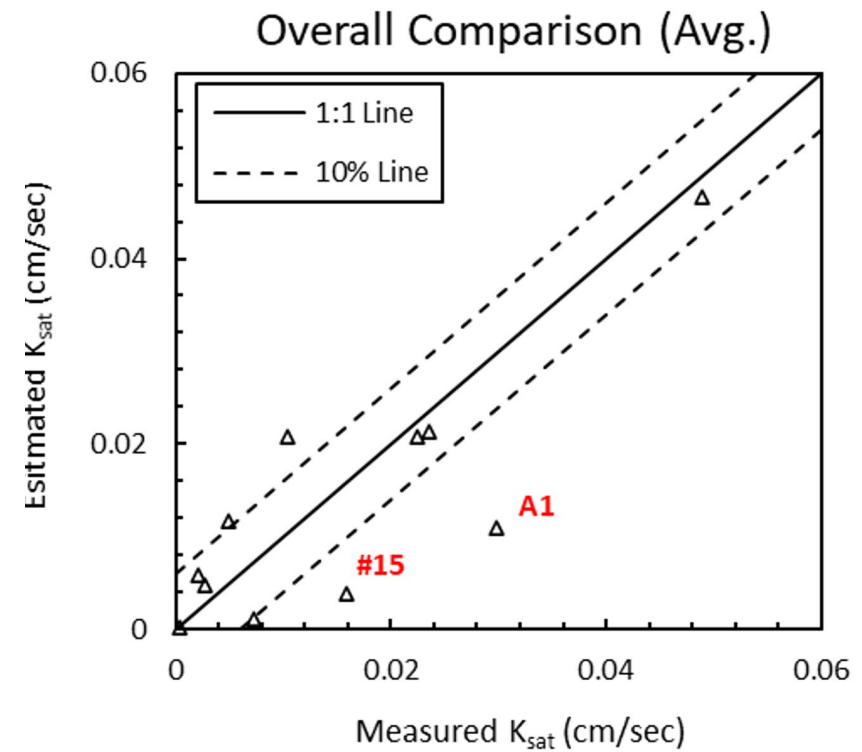
Chapuis (2004)



Empirical Equation Performance



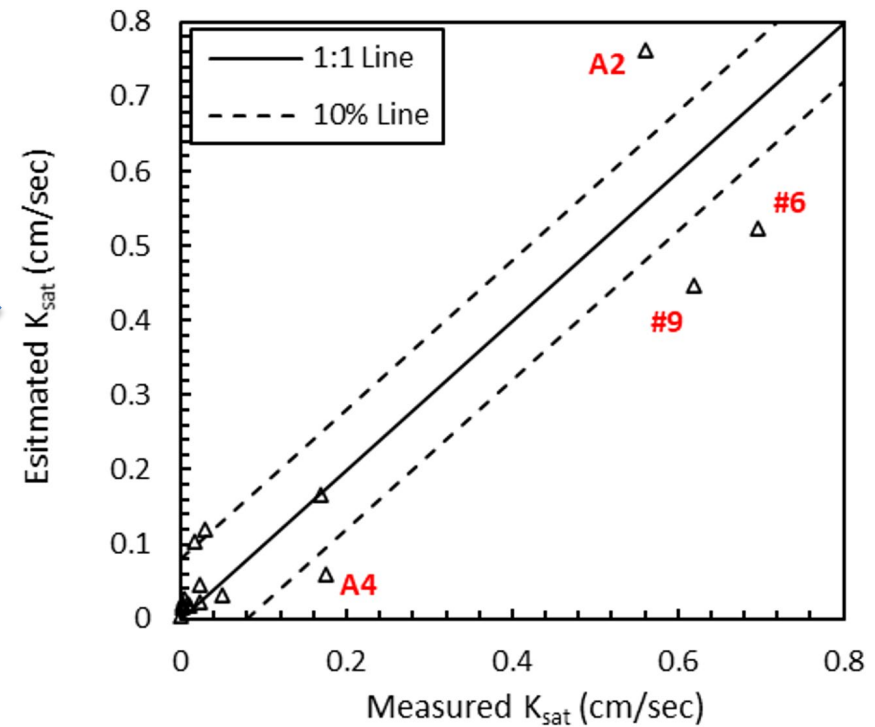
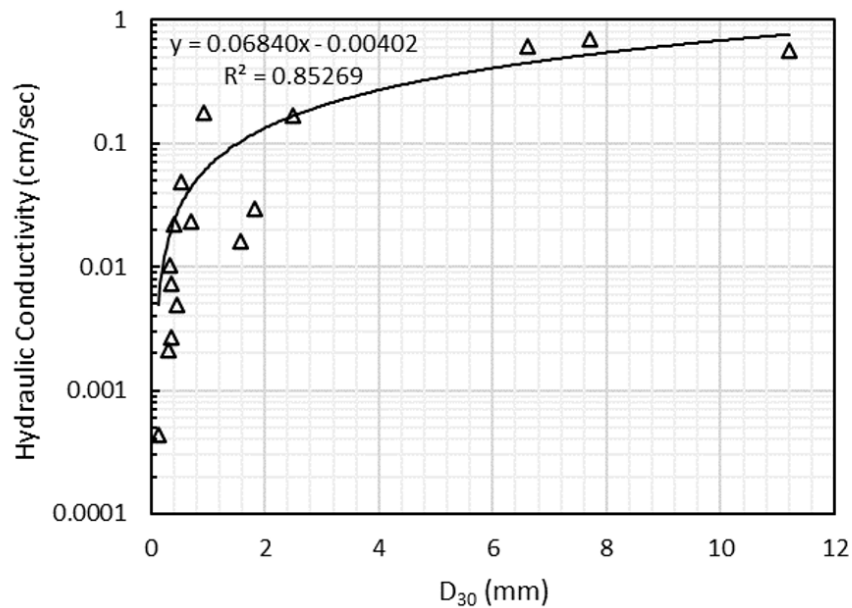
(All Samples)



(Gravels Excluded)

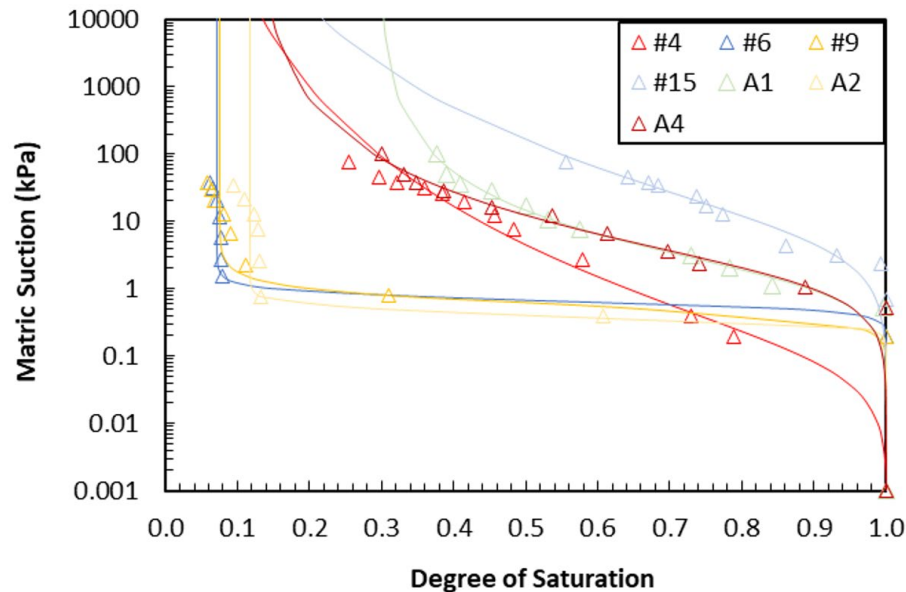
Dataset Specific Estimations

(Example using D₃₀)

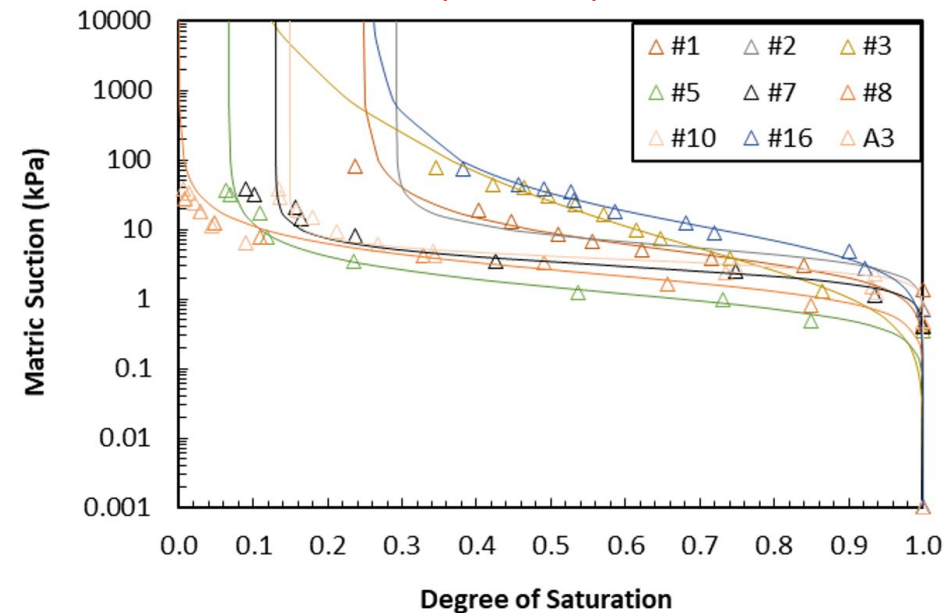


Soil-Water Characteristic Curves

(Gravels)



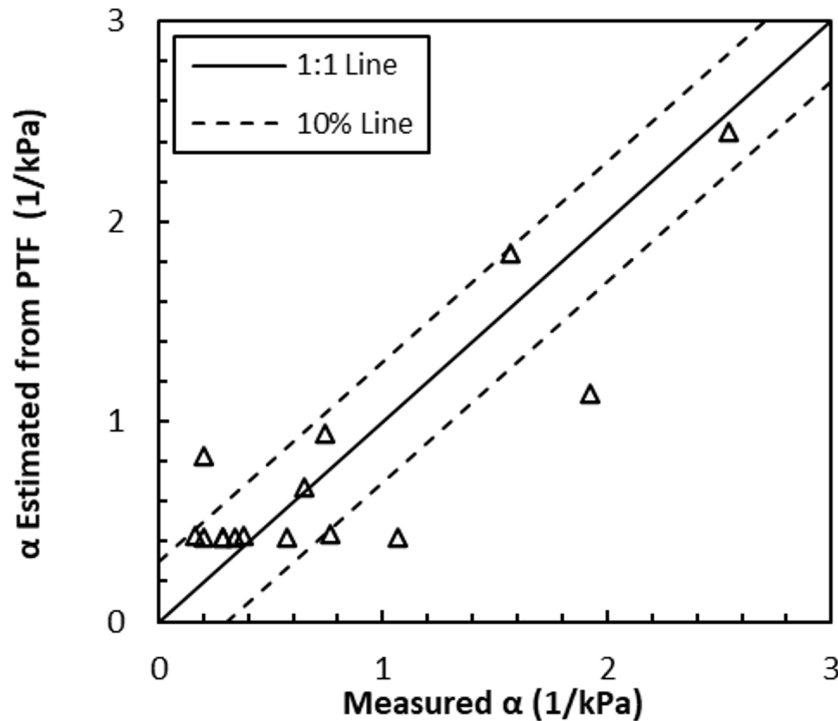
(Sands)



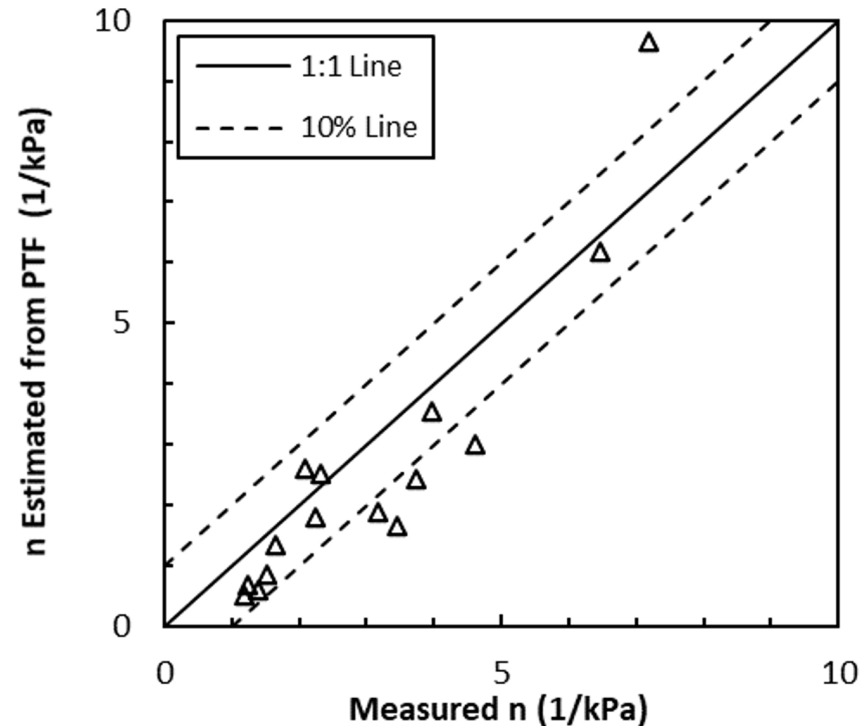
$$\Theta = S_e = \left[\frac{1}{1 + (a\psi)^n} \right]^m$$

Van Genuchten (1980) Model

Estimations of van Genuchten SWCC parameters (preliminary)



α parameter estimated using D50



n parameter estimated using D50

